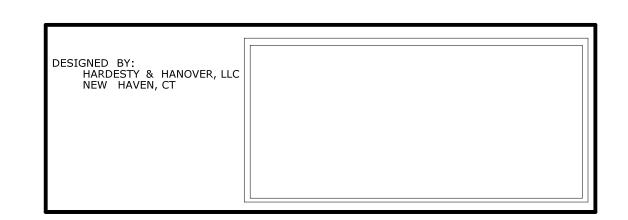
03.04 - STRUCTURAL INDEX OF DRAWINGS

DRAWING NUMBER	DRAWING TITLE	DRAWING NUMBER	DRAWING TITLE
S-01	STRUCTURAL INDEX OF DRAWINGS	S-21	DECK PATCHING PLAN - 2
S-02	GENERAL PLAN AND ELEVATION	S-22	UNDERSIDE DECK PATCHING PLAN - 1
S-03	TYPICAL SECTIONS AND NOTES	S-23	UNDERSIDE DECK PATCHING PLAN - 2
S-04	SUBSTRUCTURE REPAIR - ABUTMENTS	S-24	DECK REPAIR DETAILS
S-05	SUBSTRUCTURE REPAIR - RETAINING WALLS	S-25	DECK JOINT SEAL DETAILS - 1
S-06	SUBSTRUCTURE REPAIR - PIERS NO. WB1 & 2	S-26	DECK JOINT SEAL DETAILS - 2
S-07	SUBSTRUCTURE REPAIR - PIERS NO. WB3 & 4	S-27	DECK END REPAIR DETAILS - 1
S-08	SUBSTRUCTURE REPAIR - PIER NO. WB5	S-28	DECK END REPAIR DETAILS - 2
S-09	SUBSTRUCTURE REPAIR - DETAILS 1	S-29	PARAPET RETROFIT
S-10	SUBSTRUCTURE REPAIR - DETAILS 2	S-30	PARAPET TRANSITION DETAILS
S-11	KEEPER BLOCK DETAILS - 1	S-31	MEDIAN BARRIER DETAILS
S-12	KEEPER BLOCK DETAILS - 2	S-32	MISCELLANEOUS DETAILS
S-13	FRAMING PLAN	S-33	PAINTING AND CONTAINMENT
S-14	STRUCTURAL STEEL REPAIRS - 1	S-34	LIGHT STANDARD ANCHORAGE ADAPTER
S-15	STRUCTURAL STEEL REPAIRS - 2	S-35	PARAPET MOUNTED SIGN SUPPORT
S-16	EXPANSION BEARING REPLACEMENT - 1		
S-17	EXPANSION BEARING REPLACEMENT - 2		
S-18	TEMPORARY SUPPORT OF STRUCTURE - 1		
S-19	TEMPORARY SUPPORT OF STRUCTURE - 2		
S-20	DECK PATCHING PLAN - 1		



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REV.	DATE	REVISION DESCRIPTION	SHEET NO.	Plotted Date: 8/9/2016	

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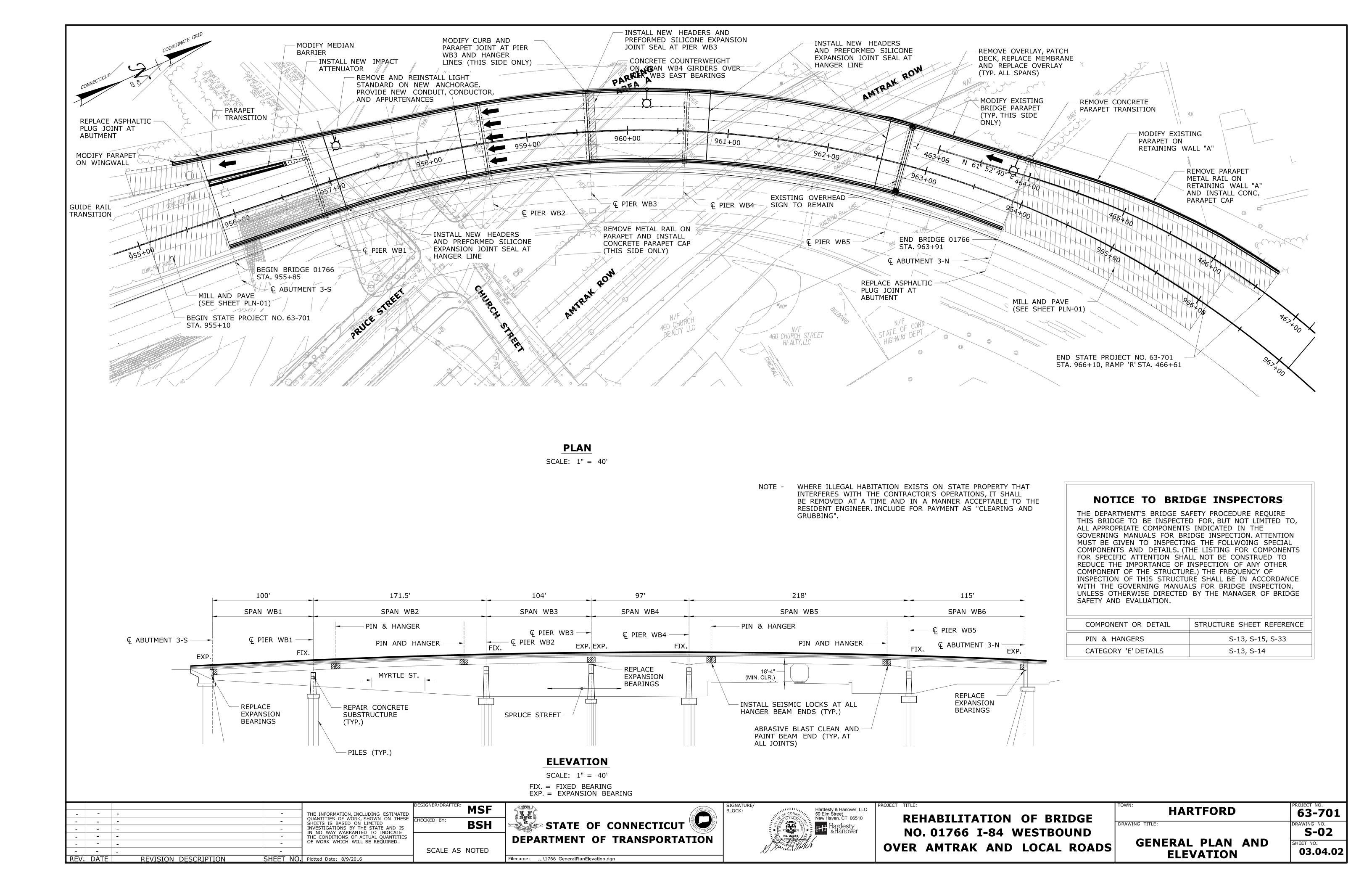


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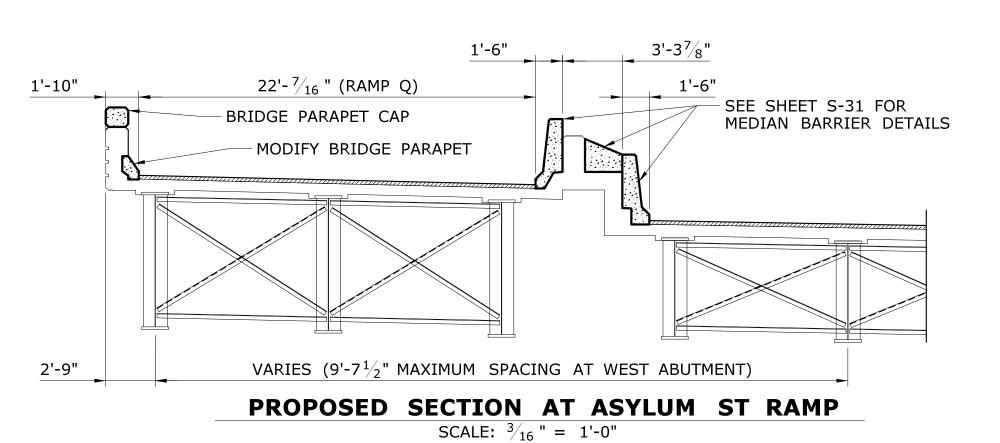


REHABILITATION OF BRIDGE	L
NO. 01766 I-84 WESTBOUND	'
OVER AMTRAK AND LOCAL ROADS	

TOWN: HARTFORD	PROJECT NO. 63-701
DRAWING TITLE:	DRAWING NO. S-01
STRUCTURAL INDEX OF DRAWINGS	03.04.01



QUANTITIES		
ITEM	UNIT	TOTAL
BIRD SPIKE	LF	2474
SURFACE PATCH (TEMPORARY)	SF	680
JACKING FOR BEARING REPLACEMENT	EA	44
CLEAN EXISTING SCUPPERS	EA	3
MODIFY SCUPPER	EA	3
EXTEND EXISTING WEEPHOLES	EA	6
REMOVAL OF EXISTING BRIDGE DRAINAGE SYSTEM	LS	1
ELASTOMERIC CONCRETE HEADERS	CF	95
ASPHALTIC PLUG EXPANSION JOINT SYSTEM	CF	180
PREFORMED JOINT SEAL	LF	390
BEARING REPLACEMENT WITH ELASTOMERIC BEARING PADS	EA	44
MODIFY BRIDGE PARAPET	LF	1096
BRIDGE PARAPET CAP	LF	1881
CLASS "S" CONCRETE	CY	10
CLASS "F" CONCRETE	CY	38
FULL DEPTH PATCH (HIGH EARLY STRENGTH CONCRETE)	CY	155
PARTIAL DEPTH PATCH	CF	4370
EPOXY INJECTION CRACK REPAIR	LF	90
DEFORMED STEEL BARS	LB	2500
DOWEL BAR SPLICER SYSTEM	EA	75
DRILLING HOLES AND GROUTING DOWELS	EA	650
CLEAN AND COAT EXPOSED REINFORCING STEEL	LF	13985
STRUCTURAL STEEL REPAIRS (SITE NO. 3)	CWT	325
TEMPORARY SUPPORT ASSEMBLY	EA	44
ABRASIVE BLAST CLEANING AND FIELD PAINTING OF BEAM ENDS (SITE NO. 3)	LS	1
CLASS 1 CONTAINMENT AND COLLECTION OF SURFACE PREPARATION DEBRIS (SITE #3)	LS	1
EMBEDDED GALVANIC ANODES	EA	180
LOCALIZED PAINT REMOVAL & FIELD PAINTING OF EXISTING STEEL	SF	703
MEMBRANE WATERPROOFING (WOVEN GLASS FABRIC)	SY	6890
RESET CONCRETE CURBING	LF	40
PROTECTIVE COMPOUND FOR BRIDGES	SY	35
TRENCHING AND BACKFILLING	FT	100
LIGHT STANDARD ANCHORAGE	EA	4
REMOVE AND REINSTALL LIGHT STANDARD	EA	4
2" FIBERGLASS CONDUIT - SURFACE MOUNTED	LF	1000



QUANTITIES ITEM UNIT TOTAL 100 2" FIBERGLASS CONDUIT IN TRENCH 16"X14"X6" NEMA 4X NON-METALLIC JUNCTION BOX NO. 10 SINGLE CONDUCTOR 3300 NO. 2 SINGLE CONDUCTOR 20 1/2" LIQUID TIGHT FLEXIBLE METAL CONDUIT 1100 NO. 8 BARE COPPER GROUNDING CONDUCTOR PARAPET MOUNTED SIGN SUPPORT

CONCRETE DI	STRIB	UTION
SUPERSTRUCTURE	C.Y.	
SUBSTRUCTURE	C.Y.	
TOTAL	C.Y.	

INSPECTION OF	FIELD	WELDS
METHOD	UNIT	QUANTITY
ULTRASONIC (UT)	L.F.	
MAGNETIC PARTICLE (MT)	L.F.	170

CONCRETE NOTES:

CLASS "F" CONCRETE: CLASS "F" CONCRETE SHALL BE USED FOR KEEPER BLOCKS, PEDESTAL REPLACEMENT, MEDIAN BARRIER REHABILITATION, AND PARAPET MODIFICATIONS AND TRANSITIONS.

CLASS "S" CONCRETE: CLASS "S" CONCRETE SHALL BE USED FOR PATCHING SUBSTRUCTURE COMPONENTS.

DECK REPAIRS: FULL DEPTH PATCHING (AS REQUIRED) AS DIRECTED BY THE ENGINEER SHALL BE PERFORMED IN ACCORDANCE WITH THE SPECIAL PROVISION "FULL DEPTH PATCH (HIGH EARLY STRENGTH CONCRETE)". PARTIAL DEPTH PATCHING (AS REQUIRED) AS DIRECTED BY THE ENGINEER SHALL BE PERFORMED IN ACCORDANCE WITH THE SPECIAL PROVISION "PARTIAL DEPTH PATCH",

JOINT SEAL: SEE SPECIAL PROVISIONS AND SHEETS S-25 AND S-26.

EXPOSED EDGES: EXPOSED EDGES OF CONCRETE SHALL BE BEVELED 1" X 1" UNLESS DIMENSIONED OTHERWISE.

CONCRETE COVER: ALL REINFORCEMENT SHALL HAVE TWO INCHES COVER UNLESS DIMENSIONED OTHERWISE.

REINFORCEMENT: ALL REINFORCEMENT SHALL BE ASTM A615 GRADE 60.

CONSTRUCTION JOINTS: CONSTRUCTION JOINTS, OTHER THAN THOSE SHOWN ON THE PLANS, WILL NOT BE PERMITTED WITHOUT THE PRIOR APPROVAL OF THE ENGINEER.

GENERAL NOTES:

SPECIFICATIONS: CONNECTICUT DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS (CONNDOT FORM 816, 2004), SUPPLEMENTAL SPECIFICATIONS DATED JANUARY 2016, AND SPECIAL PROVISIONS.

DESIGN SPECIFICATIONS: AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 2012 6TH EDITION AND AS SUPPLEMENTED BY THE CONNECTICUT DEPARTMENT OF TRANSPORTATION BRIDGE DESIGN MANUAL (2003) WITH REVISIONS DATED 2011.

BASIC ALLOWABLE DESIGN STRESSES: CLASS 'F' CONCRETE BASED ON f'c = 4,000 PSICLASS 'S' CONCRETE BASED ON f'c = 3,000 PSIREINFORCEMENT (ASTM A615 GRADE 60) fy = 60,000 PSISTRUCTURAL STEEL (ASTM A709, GRADE 50 T2) fy = 50,000 PSI

THE SPECIFIED CONCRETE STRENGTH USED IN DESIGN, f'c, OF CONCRETE COMPONENTS IS NOTED ABOVE. THE MINIMUM COMPRESSIVE STRENGTH OF CONCRETE IN THE CONSTRUCTED COMPONENTS SHALL CONFORM TO THE REQUIREMENTS OF "SECTION 6.01 CONCRETE FOR STRUCTURES".

LIVE LOAD: HL-93

FUTURE PAVING ALLOWANCE: NONE

STRUCTURAL STEEL: SEE DWG. NO. S-13 FOR EXISTING DESIGNATIONS AND REQUIREMENTS. SEE DWG. NO. S-14 FOR NEW DESIGNATIONS AND REQUIREMENTS.

PAINT: PAINT SHALL CONFORM TO THE REQUIREMENTS OF THE SPECIAL PROVISIONS, "ABRASIVE BLAST CLEANING AND FIELD PAINTING OF BEAM ENDS (SITE NO. 1)." THE COLOR OF THE TOPCOAT MATERIAL ON THE STRUCTURAL STEEL SHALL CONFORM TO FEDERAL STANDARD COLOR NO. 25240 (BLUE GREY).

PAINTING LIMITS: SEE DWG. NO. S-33 FOR PAINTING LIMITS AND CONTAINMENT

BITUMINOUS CONCRETE OVERLAY: PROPOSED WEARING SURFACE SHALL CONSIST OF TWO (2) LIFTS. THE FIRST SHALL BE HMA S.025 (1" THICK). THE SECOND SHALL BE HMA S0.375 (1.5" THICK). PAVEMENT THICKNESS SHALL BE TAPERED AT ENDS TO MATCH PAVEMENT THICKNESS AT CONSTRUCTION LIMITS. SEE HIGHWAY

DIMENSIONS: WHEN DECIMALS ARE GIVEN TO LESS THAN THREE DECIMAL PLACES, THE OMITTED DIGITS SHALL BE ASSUMED ZEROES.

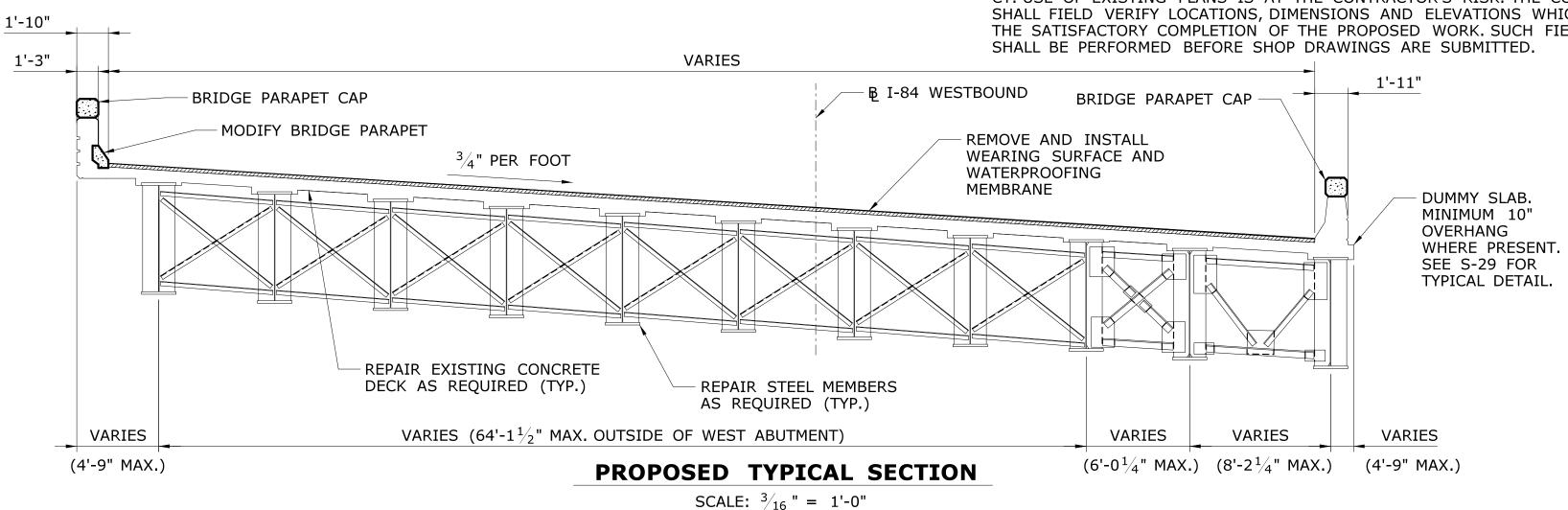
EXISTING DIMENSIONS: DIMENSIONS OF THE EXISTING STRUCTURE SHOWN ON THESE PLANS ARE FOR GENERAL REFERENCE ONLY. THEY HAVE BEEN TAKEN FROM THE ORIGINAL DESIGN DRAWINGS AND ARE NOT GUARANTEED. THE CONTRACTOR SHALL TAKE ALL FIELD MEASUREMENTS NECESSARY TO ASSURE PROPER FIT OF THE FINISHED WORK AND SHALL ASSUME FULL RESPONSIBILITY FOR THEIR ACCURACY. WHEN SHOP DRAWINGS BASED ON FIELD MEASUREMENTS ARE SUBMITTED FOR APPROVAL, THE FIELD MEASUREMENTS SHALL BE INDICATED ON THE DRAWINGS AND SHALL ALSO BE SUBMITTED FOR REFERENCE BY THE REVIEWER.

ELEVATIONS: ELEVATIONS SHOWN ARE TAKEN FROM THE ORIGINAL CONTRACT PLAN DRAWINGS UNLESS NOTED OTHERWISE.

TRAFFIC: ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE SPECIAL PROVISIONS, "MAINTENANCE AND PROTECTION OF TRAFFIC" AND "PROSECUTION AND PROGRESS" AND CONSTRUCTION STAGES SHOWN ON THE PLANS.

UTILITIES: THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF ALL UTILITIES WITHIN THE PROJECT LIMITS PRIOR TO THE START OF CONSTRUCTION AND SHALL TAKE NECESSARY PRECAUTIONS WHEN WORKING NEAR UTILITIES SO AS TO NOT DISTURB THEM OR PLACE ANY LOAD OR EQUIPMENT ON THEM. ALL UTILITY COMPANIES SHALL BE NOTIFIED 48 HOURS PRIOR TO ANY WORK AFFECTING CABLES CONDUITS, OR OTHER UTILITIES.

EXISTING PLANS: INFORMATION PERTAINING TO THE EXISTING STRUCTURE IS BASED ON EXISTING PLANS, WHICH ARE AVAILABLE FOR INSPECTION AT THE CONNECTICUT DEPARTMENT OF TRANSPORTATION PLANS OFFICE, 160 PASCONE PLACE, NEWINGTON, CT. USE OF EXISTING PLANS IS AT THE CONTRACTOR'S RISK, THE CONTRACTOR SHALL FIELD VERIFY LOCATIONS, DIMENSIONS AND ELEVATIONS WHICH CAN AFFECT THE SATISFACTORY COMPLETION OF THE PROPOSED WORK. SUCH FIELD VERIFICATION



THE INFORMATION, INCLUDING ESTIMATED QUANTITIES OF WORK, SHOWN ON THESE - | - | -SHEETS IS BASED ON LIMITED INVESTIGATIONS BY THE STATE AND IS - | - | -N NO WAY WARRANTED TO INDICATE _ | - | -THE CONDITIONS OF ACTUAL QUANTITIES OF WORK WHICH WILL BE REQUIRED. - - -REV. DATE REVISION DESCRIPTION SHEET NO. Plotted Date: 8/9/2016

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SCALE AS NOTED

STATE OF CONNECTICUT **DEPARTMENT OF TRANSPORTATION**

Filename: ...\1766 Typical Section.dgn

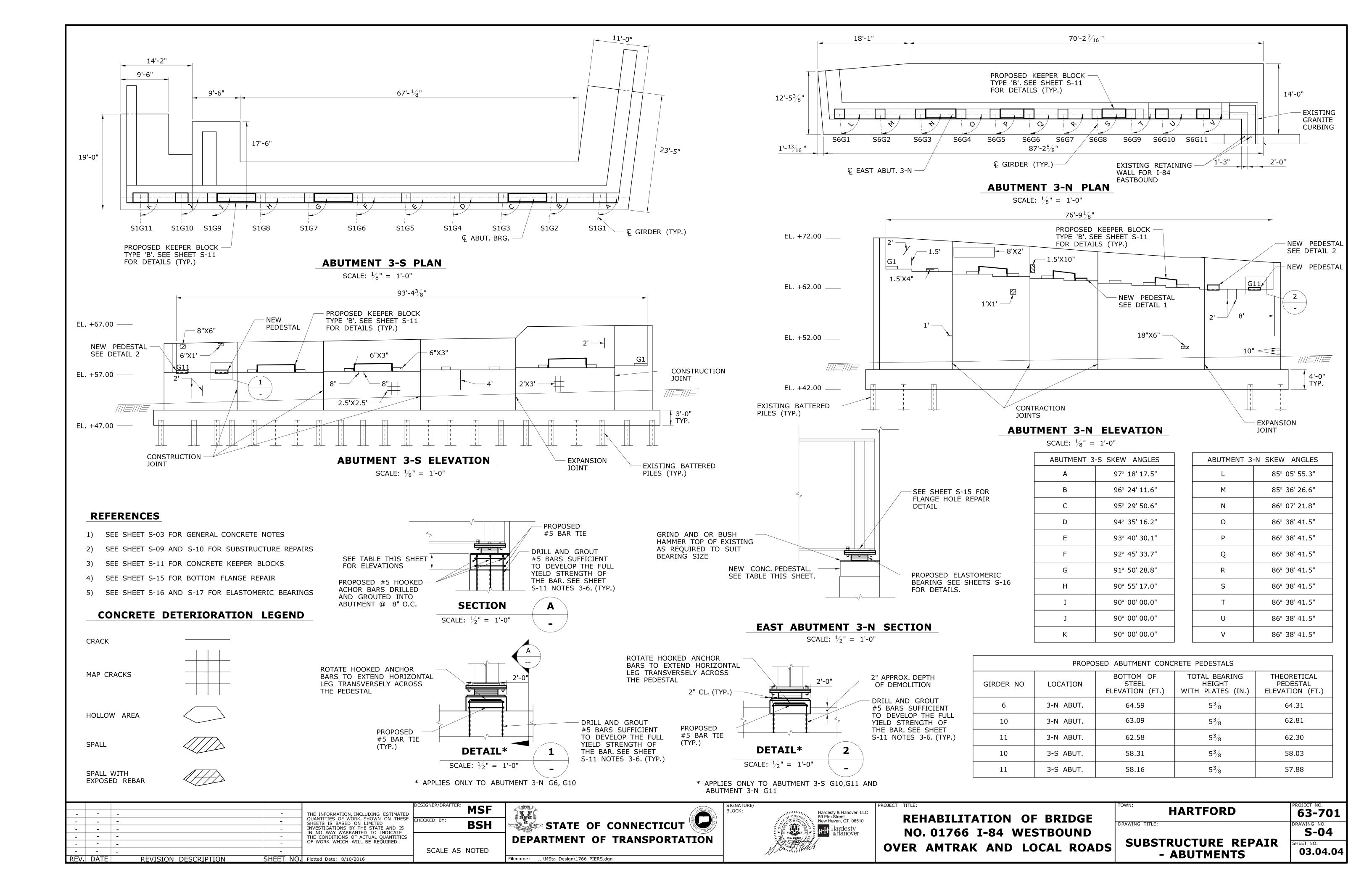


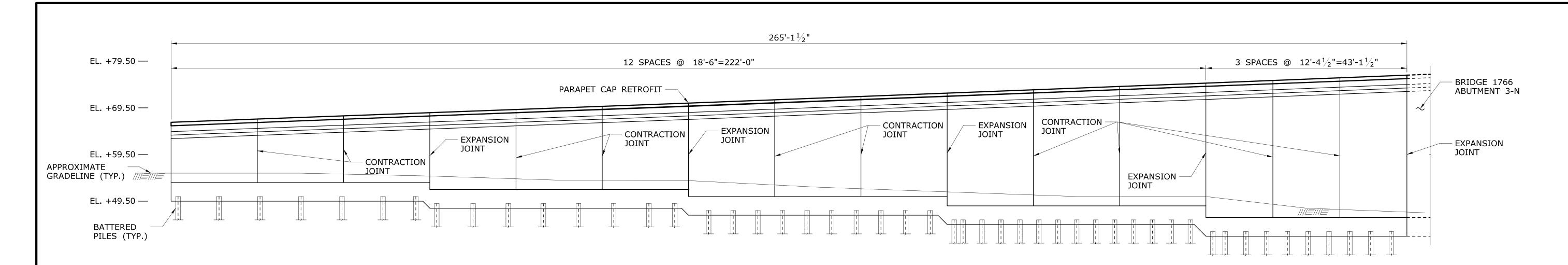
REHABILITATION OF BRIDGE NO. 01766 I-84 WESTBOUND **OVER AMTRAK AND LOCAL ROADS**

HARTFORD 63-701 DRAWING TITLE: **S-03**

03.04.03

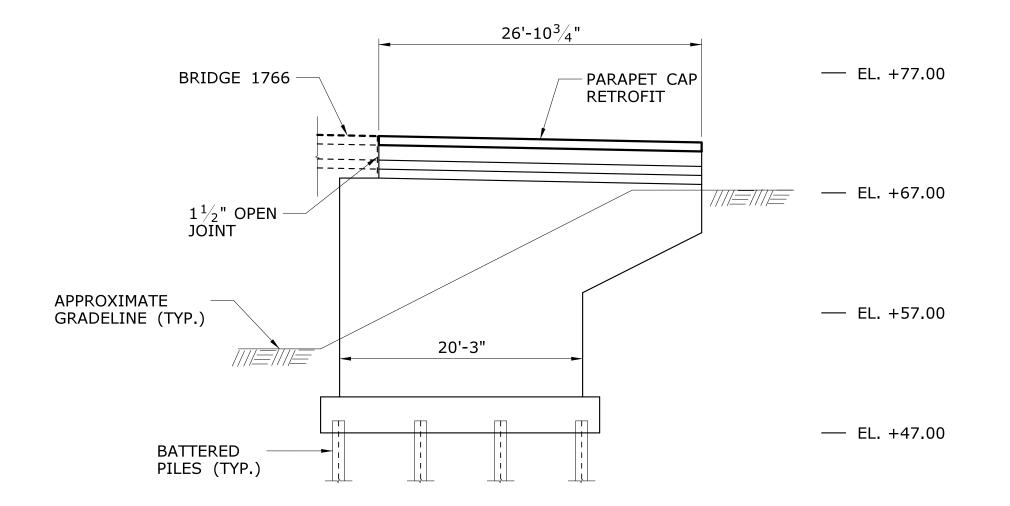
TYPICAL SECTIONS **AND NOTES**



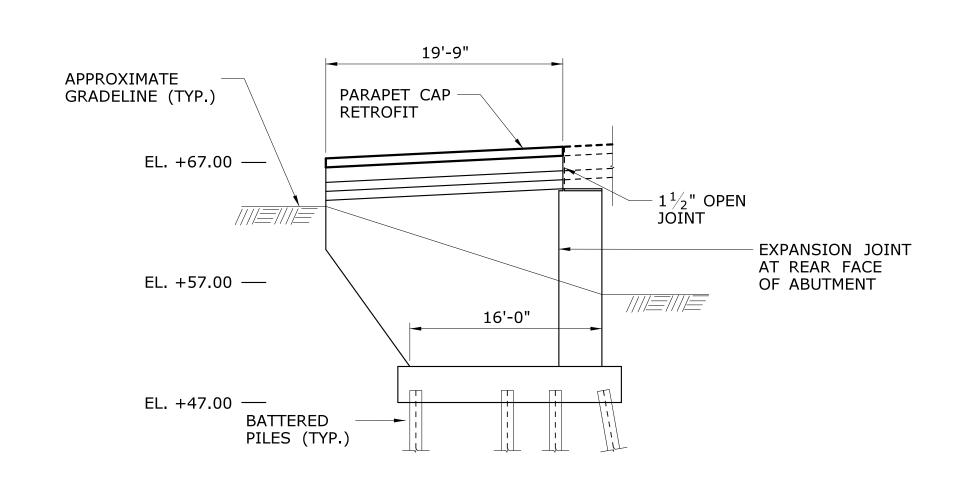


ABUTMENT 3-N NORTH WINGWALL

SCALE: $\frac{3}{32}$ " = 1'-0"



ABUTMENT 3-S NORTH WINGWALL SCALE: 1/8" = 1'-0"



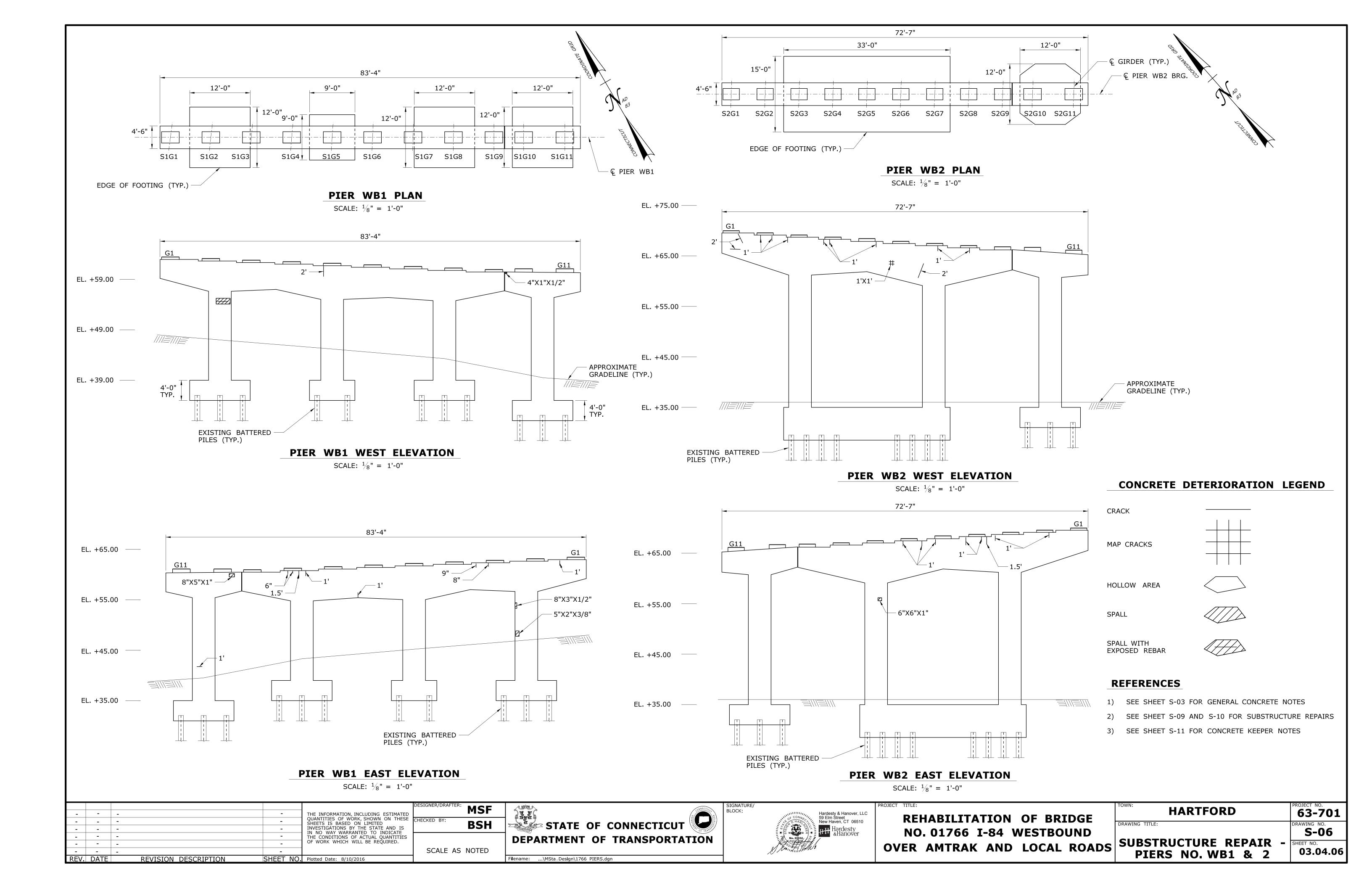
ABUTMENT 3-S SOUTH WINGWALL

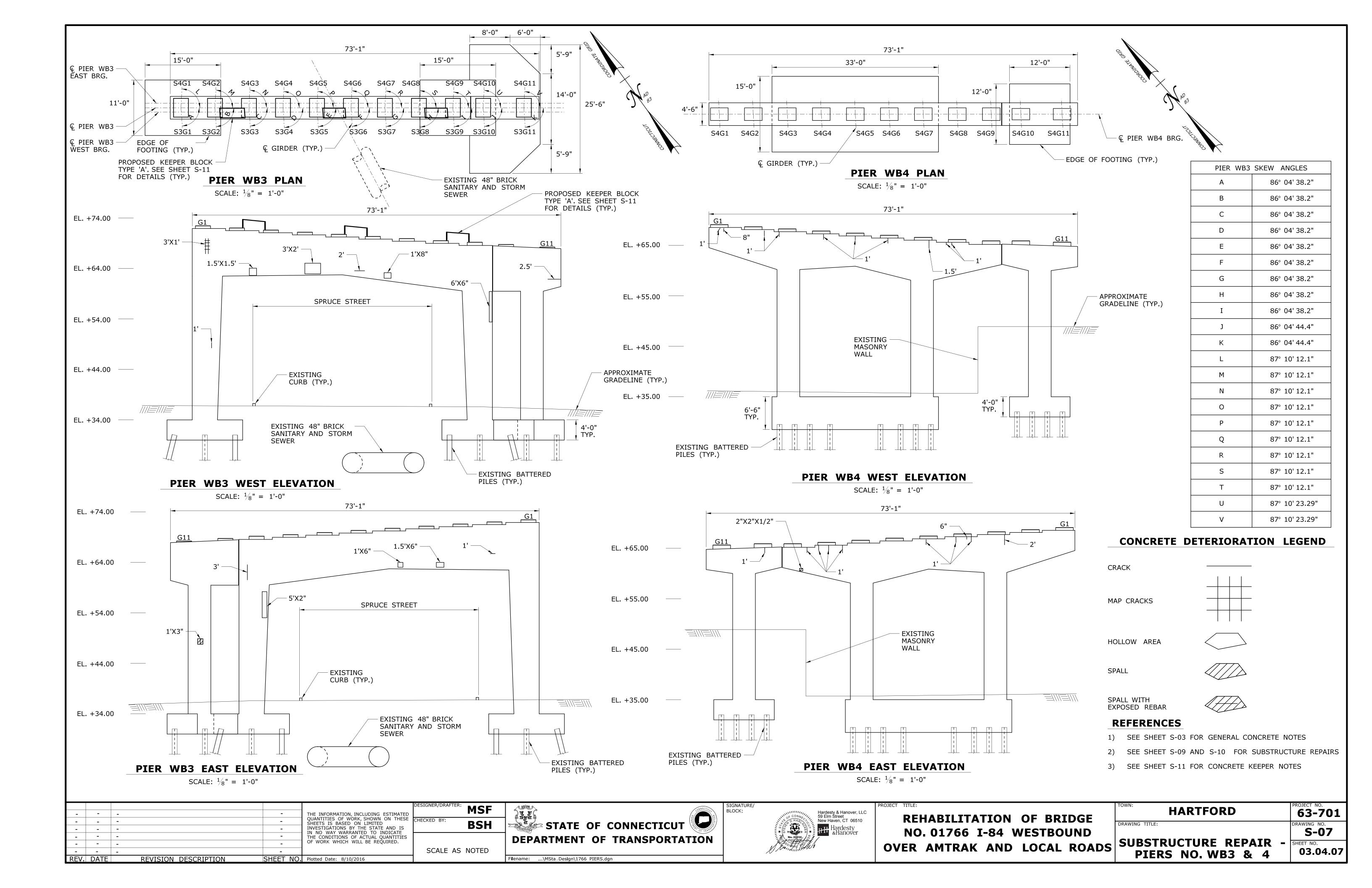
SCALE: $\frac{1}{8}$ " = 1'-0"

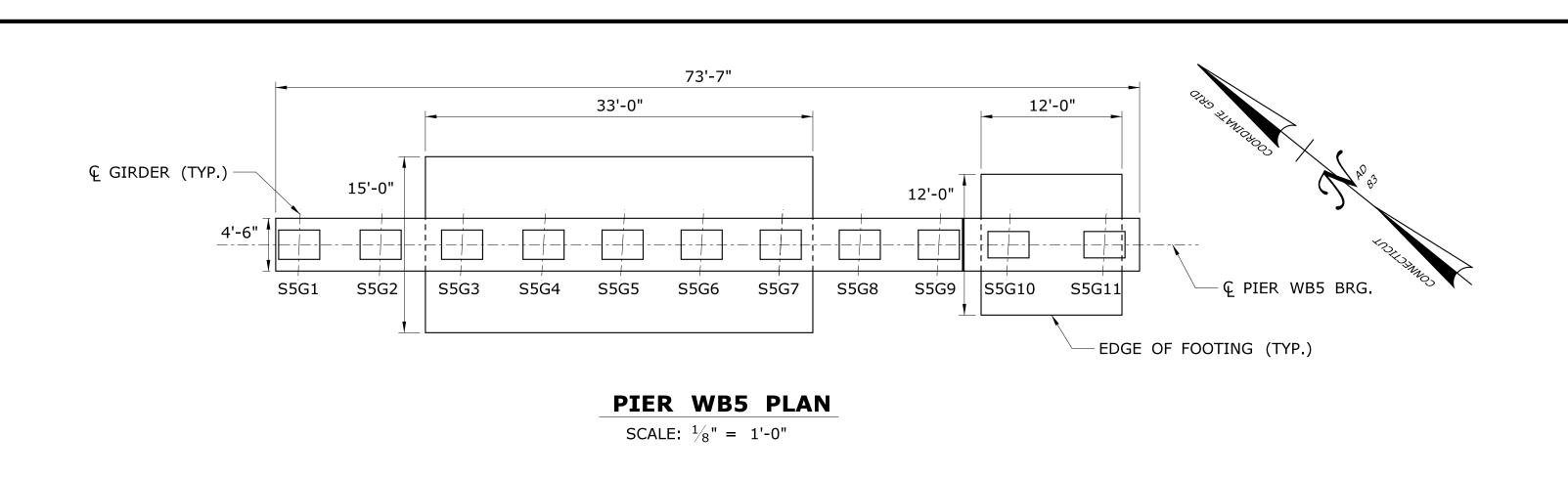
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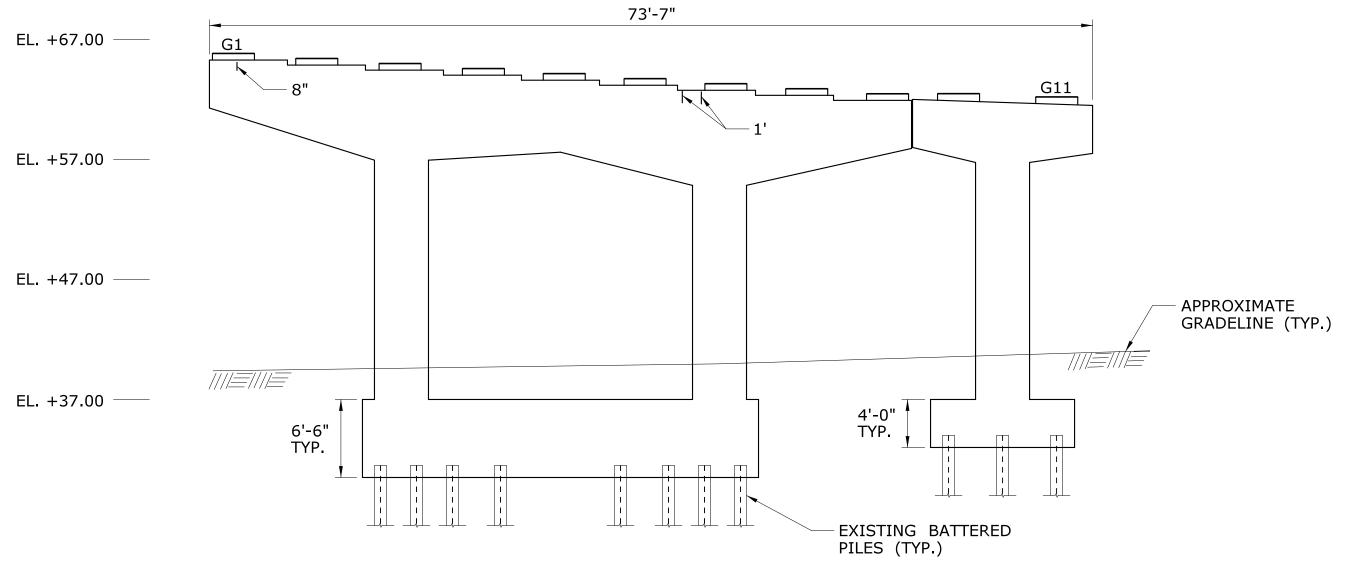
- 1) SEE SHEET S-03 FOR GENERAL CONCRETE NOTES
- 2) SEE SHEET S-29 FOR PARAPET RETROFIT

	- THE INFORMATION, INCLUDING ESTIMATED	DESIGNER/DRAFTER: MSF	S TONNECTICOS	SIGNATURE/ BLOCK: Hardesty & Hanover, LI	PROJECT TITLE:	HARTFORD	PROJECT NO. 63-701
	QUANTITIES OF WORK, SHOWN ON THESE SHEETS IS BASED ON LIMITED INVESTIGATIONS BY THE STATE AND IS	CHECKED BY: BSH	STATE OF CONNECTICUT	New Haven, CT 06510	REHABILITATION OF BRIDGE	DRAWING TITLE:	DRAWING NO.
	IN NO WAY WARRANTED TO INDICATE THE CONDITIONS OF ACTUAL QUANTITIES		DEPARTMENT OF TRANSPORTATION	* 5	NO. 01766 I-84 WESTBOUND		S-05
	OF WORK WHICH WILL BE REQUIRED.	SCALE AS NOTED	DEPARTMENT OF TRANSPORTATION	Just Jan	OVER AMTRAK AND LOCAL ROAD	SUBSTRUCTURE REPAIR	SHEET NO. 03.04.0
REVIDATE REVISION DESCRIPTION	SHEET NO Plotted Date: 8/10/2016	 	Filename: \MSta Design\1766 PIERS don	-		- RETAINING WALLS	03.04.0



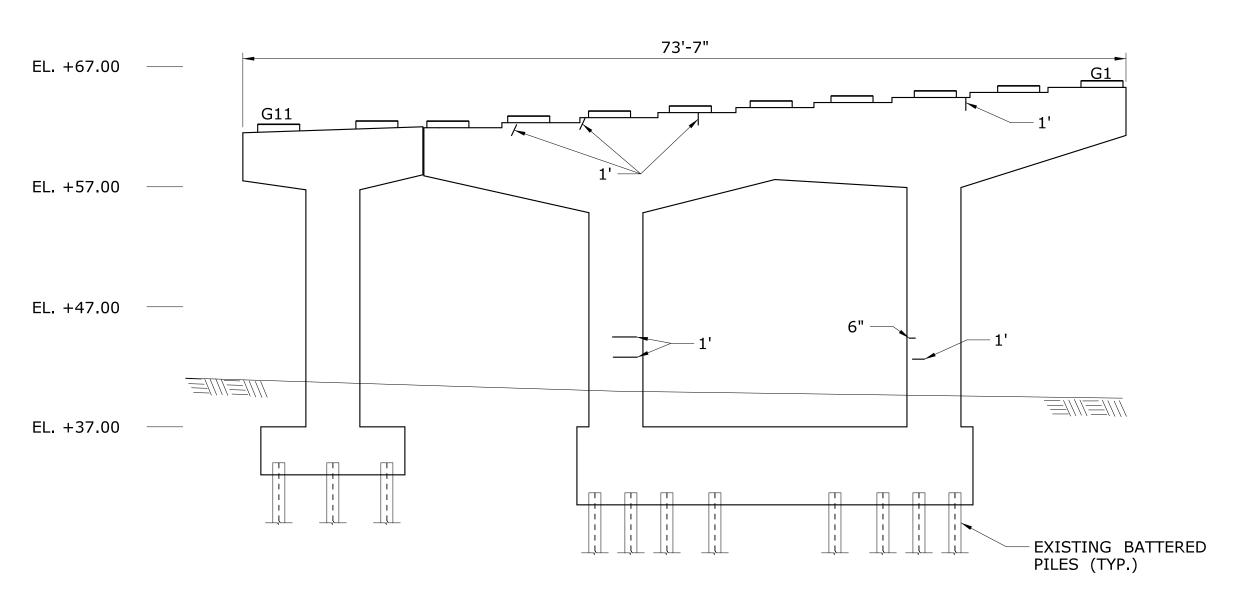






PIER WB5 WEST ELEVATION

SCALE: $\frac{1}{8}$ " = 1'-0"



PIER WB5 EAST ELEVATION

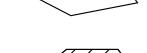
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CONCRETE DETERIORATION LEGEND

CRACK

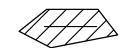


HOLLOW AREA



SPALL WITH EXPOSED REBAR

SPALL

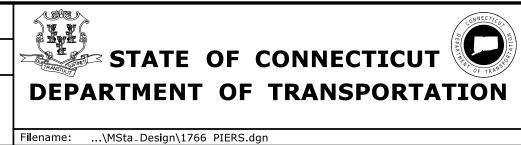


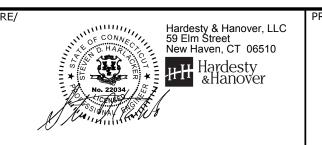
REFERENCES

- 1) SEE SHEET S-03 FOR GENERAL CONCRETE NOTES
- 2) SEE SHEET S-09 AND S-10 FOR SUBSTRUCTURE REPAIRS

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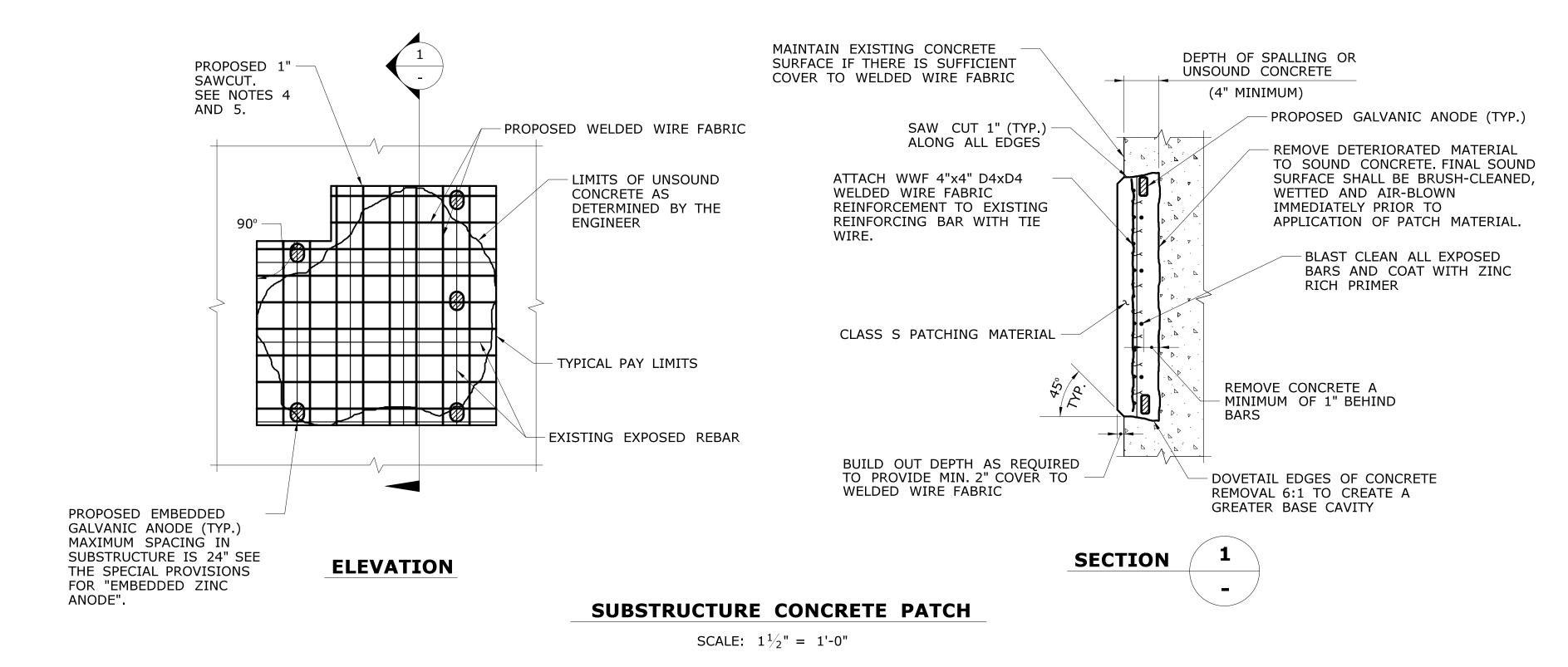
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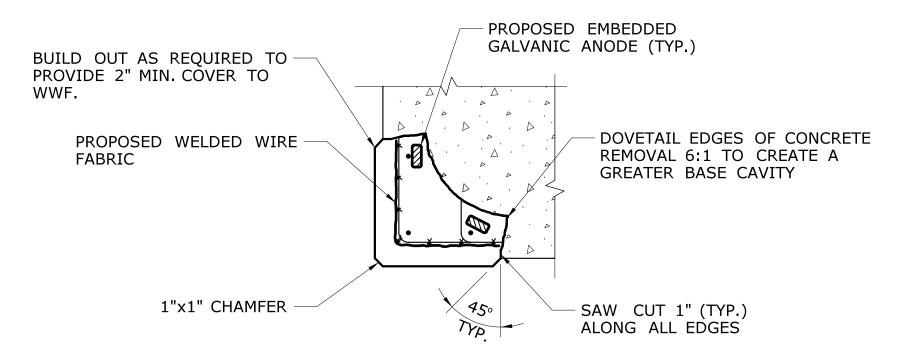




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	REHABILITATION OF BRIDGE
	NO. 01766 I-84 WESTBOUND
OV	ER AMTRAK AND LOCAL ROADS

	HARTFORD	PROJECT NO. 63-701
	DRAWING TITLE:	DRAWING NO. S-08
S	SUBSTRUCTURE REPAIR - PIER NO. WB5	SHEET NO. 03.04.08





CORNER PATCH DETAIL

SCALE: $1\frac{1}{2}$ " = 1'-0"

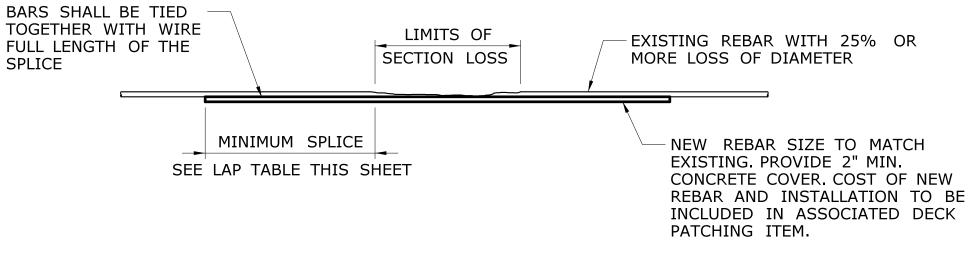
NOTE

TYPICAL DETAIL APPLICABLE FOR COLUMN CORNERS, OVERHEAD CORNERS, AND TOP EDGES OF PIER CAPS. WORK WITH "SUBSTRUCTURE CONCRETE PATCH" DETAILS AND PROCEDURE NOTES ON THIS SHEET.

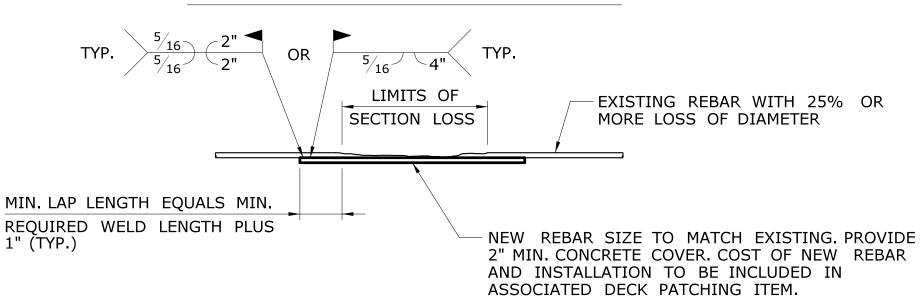
REBAR LAP TABLE					
BAR SIZE	MINIMUM LAP LENGTH				
	INCH				
#4	15"				
#5	18"				

**THE ENGINEER SHALL BE NOTIFIED OF ANY BARS GREATER THAN #5 THAT REQUIRE REPAIR. THESE BARS SHALL BE SPLICED AT THE DIRECTION OF THE ENGINEER.

SHEET NO. Plotted Date: 8/9/2016



LAPPED TIED SPLICE REBAR



LAPPED WELDED SPLICE DETAIL

REINFORCEMENT SPLICE DETAILS

NOT TO SCALE

SPLICE NOTES

- 1. WELDED SPLICE DETAIL TO BE USED ONLY IF IT IS VERIFIED THAT EXISTING STEEL IS WELDABLE BASED ON ITS CHEMICAL COMPOSITION.
- 2. WELDING SHALL BE DONE IN ACCORDANCE WITH AWS D1.4 STRUCTURAL WELDING CODE REINFORCING STEEL.
- 3. MECHANICAL SPLICERS ARE AN ACCEPTABLE ALTERNATE IF APPROVED BY THE ENGINEER.

SUBSTRUCTURE CONCRETE PATCH REPAIR PROCEDURE

- A. THE SUBSTRUCTURE CONCRETE PATCH DETAIL APPLIES TO DETERIORATED AREAS OF REINFORCED CONCRETE WHERE REINFORCING BARS ARE EXPOSED.
- B. REMOVE DETERIORATED MATERIAL TO SOUND CONCRETE LEAVING NO OFFSET OR ABRUPT CHANGES IN CONTOUR. REMOVE CONCRETE A MINIMUM OF 1" BEYOND THE EXPOSED REINFORCING.
- C. CLEAN EXISTING REINFORCING STEEL AND CONCRETE (NEWLY EXPOSED) PER THE REQUIREMENTS OF THE SPECIAL PROVISION. MISSING OR DETERIORATED REINFORCING STEEL SHALL BE REPLACED AND SPLICED AS SHOWN IN DETAIL OR AS DIRECTED BY THE ENGINEER. COST OF REINFORCING STEEL SPLICING IS INCIDENTAL TO THE ITEM "CLASS S CONCRETE".
- D. INSTALL GALVANIC ANODES AND WELDED WIRE FABRIC. APPLY ZINC RICH PRIMER TO EXISTING AND NEW REINFORCING STEEL IMMEDIATELY PRIOR TO PLACING PATCHING CONCRETE. WELDED WIRE FABRIC AND ZINC COATING COST INCIDENTAL TO THE ITEM "CLASS S CONCRETE".
- E. FORM AND PATCH SURFACE.
- F. ALL NEW EXPOSED CONCRETE SURFACES WITHIN AREA TO BE REPAIRED SHALL BE RUBBED TO PRODUCE A SMOOTH FINISH.
- G. ZINC ANODES TO BE INSTALLD IN ALL PATCHES. ANODES SHALL BE PAID FOR AS "EMBEDDED GALVANIC ANODES" AND SHALL BE INSTALLED PER THE REQUIREMENTS OF THE SPECIAL PROVISIONS.

 MAXIMUM ANODE SPACING SHALL BE 20" ON CENTER.

SUBSTRUCTURE REPAIR NOTES

- 1. THE CONTRACTOR SHALL REPAIR THE SUBSTRUCTURE DEFICIENCIES IDENTIFIED ON PLAN DRAWINGS S-04 THROUGH S-08. REPAIR DETAILS APPLY TO SPALLED, SCALED, AND HOLLOW AREAS IN ABUTMENTS AND PIERS WHERE REQUIRED AND NOTED ON DRAWINGS AND AS DIRECTED BY THE ENGINEER.
- 2. ESTABLISH LIMITS OF REPAIRS AS SHOWN AND AT THE DIRECTION OF THE ENGINEER. THE EXTENT AND LOCATION OF ALL CONCRETE SUBSTRUCTURE REPAIRS ARE TO BE FIELD VERIFIED AND APPROVED BY THE ENGINEER AFTER THE CONTRACTOR HAS SOUNDED AND MARKED OUT THE REPAIR AREAS.
- 3. SUBSTRUCTURE CONCRETE PATCH REPAIRS SHALL BE PAID FOR UNDER THE ITEM "CLASS 'S' CONCRETE".
- 4. THE LIMITS OF THE REPAIRS SHALL BE SAWCUT ALONG NEAT LINES WHERE PRACTICAL TO A DEPTH OF 1" TO PRODUCE A CLEAN EDGE. SEE SPECIAL PROVISIONS.
- NEW CONCRETE PATCHES SHALL MATCH SHAPE OF EXISTING CONCRETE SURFACES. REPAIR CONFIGURATIONS SHOULD BE KEPT AS SIMPLE AS POSSIBLE, PREFERABLY WITH SQUARE CORNERS. COLOR OF NEW PATCH CONCRETE SHALL MATCH COLOR OF THE ADJACENT SURFACES AS CLOSELY AS POSSIBLE.
- 6. EXPOSED REINFORCING BARS SHALL BE BLAST CLEANED AND COATED WITH A SINGLE COMPONENT ZINC RICH PRIMER THAT CONFORMS TO THE SPECIAL PROVISIONS, BEFORE APPLYING THE PATCHING MATERIAL COST OF PRIMER SHALL BE INCLUDED IN THE COST FOR "CLASS 'S' CONCRETE". INSTALL EMBEDDED GALVANIC ANODES PRIOR TO APPLYING PATCHING MATERIAL.
- 7. SPLICED REINFORCING BARS SHALL BE COATED WITH A SINGLE COMPONENT ZINC RICH PRIMER THAT CONFORMS TO THE SPECIAL PROVISIONS BEFORE APPLYING PATCHING MATERIAL. COST OF PRIMER SHALL BE INCLUDED IN THE COST FOR "CLASS 'S' CONCRETE".
- 8. THE SURFACE OF EXISTING OR PREVIOUSLY CAST CONCRETE SHALL BE BLAST CLEANED, ROUGHENED AND WETTED WITH CLEAN WATER BEFORE NEW CONCRETE IS PLACED PER THE SPECIAL PROVISIONS.
- 9. EXISTING CRACKS IDENTIFIED BY THE ENGINEER SHALL BE SEALED IN ACCORDANCE WITH THE SPECIAL PROVISIONS AND THE CRACK REPAIR DETAILS SHOWN ON SHEET S-12.
- 10. COVER OVER EXISTING REINFORCEMENT SHALL BE A MINIMUM OF 2". FACE OF PATCHED AREA MAY BE BUILT OUT TO MEET THIS REQUIREMENT, IF NECESSARY.
- 11. THE REMOVAL OF DETERIORATED CONCRETE SHALL PROCEED AS DIRECTED BY THE ENGINEER. IF THE REMOVAL OF DETERIORATED CONCRETE BECOMES EXCESSIVE, THE REMOVAL WORK SHALL BE STOPPED AT THE LOCATION AND THE ENGINEER NOTIFIED IMMEDIATELY. COST OF REMOVAL OF DETERIORATED CONCRETE AND SURFACE PERPARATION OF THE REPAIR AREA SHALL BE INCLUDED IN ITEM "CLASS 'S' CONCRETE".
- 12. THE CONTRACTOR SHALL NOT REMOVE CONCRETE EXCEPT IN THE PRESENCE OF THE ENGINEER OR HIS APPOINTED REPRESENTATIVE. IF THE AREA REMOVED EXCEEDS 20 SQUARE FEET, OR MORE THAN 30% OF COLUMN CROSS SECTIONAL PERIMETER, OR IF THE REMOVAL DEPTH EXTENDS MORE THAN 1-½" BEHIND THE MAIN REINFORCING BARS, THE CONTRACTOR SHALL CEASE REMOVAL OPERATIONS AND NOTIFY THE ENGINEER IMMEDIATELY. THE ENGINEER SHALL DETERMINE IF THE REMOVAL OPERATIONS REDUCE THE STRUCTURAL CAPCAITY OF THE ELEMENT.
- 13. SHALLOW CONCRETE DETERIORATION REMOVED TO SOUND CONCRETE AND NOT EXPOSING EXISTING REINFORCING STEEL SHALL NOT BE PATCHED.
- 14. THE CONTRACTOR SHALL PROVIDE INSPECTION ACCESS TO THE RESIDENT ENGINEER DURING THE PERFORMANCE OF THIS WORK AT NO ADDITIONAL COST TO THE STATE.

-	-	-	_	THE INFORMATION, INCLUDING ESTIMATED
-	-	-	-	QUANTITIES OF WORK, SHOWN ON THESE SHEETS IS BASED ON LIMITED
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-	-	-	-	THE CONDITIONS OF ACTUAL QUANTITIES
_	-	-	-	OF WORK WHICH WILL BE REQUIRED.
-	-	_	_	

REVISION DESCRIPTION

REV. DATE

BSH
D BY:
SDH

SCALE AS NOTED





REHABILITATION OF BRIDGE NO. 01766 I-84 WESTBOUND OVER AMTRAK AND LOCAL ROADS

ND
SOADS SUBS

HARTFORD

G TITLE:

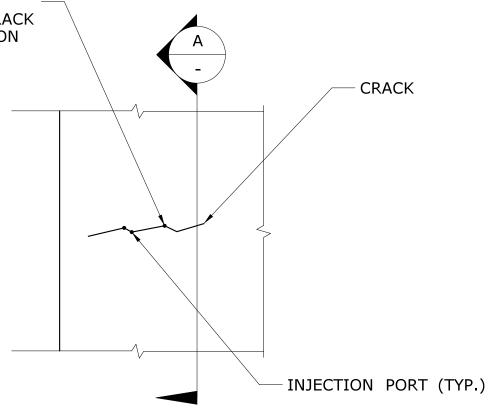
PROJECT NO.
63-701

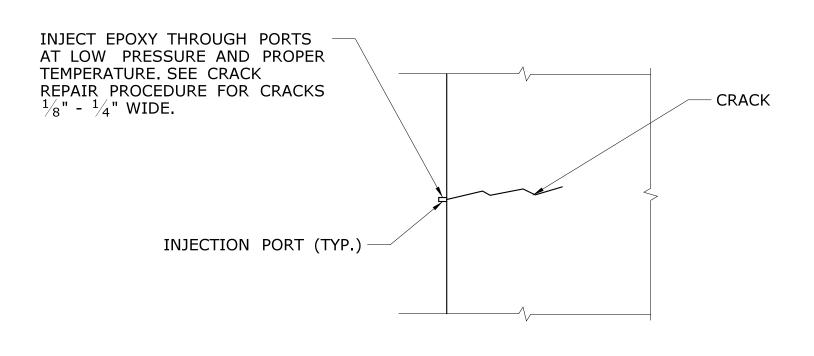
DRAWING NO.
S-09

SUBSTRUCTURE REPAIR
- DETAILS 1

03.04.09

CLEAN CRACK, INSTALL INJECTION PORTS AND SEAL SURFACE OF CRACK BETWEEN PORTS BEFORE INJECTION GROUTING





SECTION

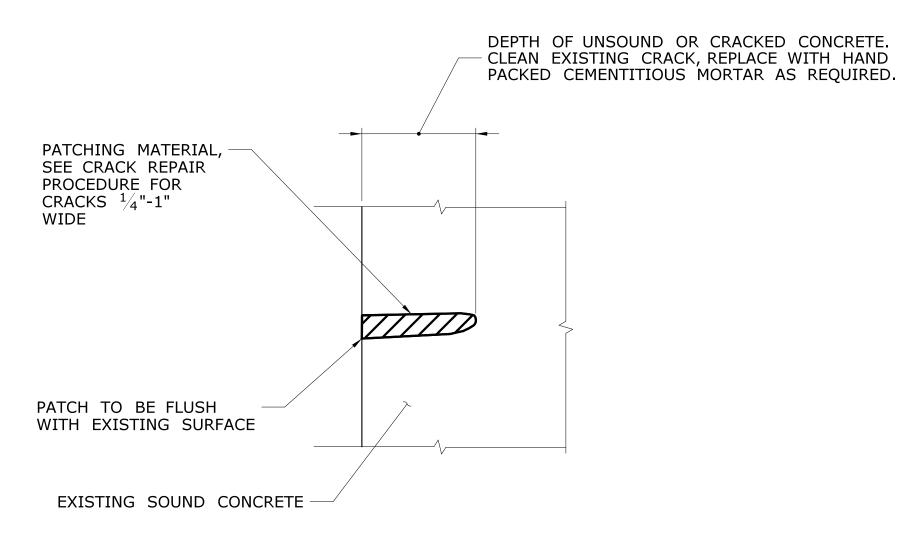
ELEVATION

CRACKS $\frac{1}{8}$ " - $\frac{1}{4}$ " WIDE

NOT TO SCALE

CRACK REPAIR PROCEDURE FOR CRACKS 1/8" - 1/4" WIDE

- 1. SURFACE PREPARATION:
 - -REMOVE DUST, LAITANCE, GREASE, IMPREGNATIONS, FOREIGN PARTICLES AND DISINTEGRATED MATERIALS. SURFACE MUST BE CLEAN AND SOUND WITH A ROUGHENED TEXTURE. IDEALLY DRY, SURFACE MAY BE DAMP BUT SHALL BE FREE OF STANDING WATER.
- 2. APPLICATION AND FINISH:
 - SET GROUT PRESSURE INJECTION PORTS INTO PLACE.
 - MIX EPOXY ADHESIVE PER MANUFACTURER'S SPECIFICATION.
 - SEAL CRACKS AND PORTS BY APPLYING MIXED EPOXY ADHESIVE MATERIAL OVER THE CRACKS TO BE PRESSURE INJECTED WITH THE HIGH-STRENGTH EPOXY GROUT.
 - MIX EPOXY GROUT PER MANUFACTURER'S SPECIFICATION.
 - WHEN THE EPOXY ADHESIVE HAS CURED, INJECT THE EPOXY GROUT WITH STEADY PRESSURE.
 - ALLOW THE INJECTED EPOXY GROUT TO SET THEN CUT THE PRESSURE INJECTION PORTS FLUSH WITH THE EPOXY ADHESIVE.
- 3. CRACK REPAIRS SHALL BE PAID UNDER THE ITEM "EPOXY INJECTION CRACK REPAIR", SEE SPECIAL PROVISIONS.
- 4. ANY CRACKS THAT MEASURE LESS THAN $\frac{1}{8}$ " AT THEIR WIDEST POINT SHALL NOT BE REPAIRED UNLESS DIRECTED BY THE ENGINEER.



CRACKS $\frac{1}{4}$ " - 1" WIDE

NOT TO SCALE

CRACK REPAIR PROCEDURE FOR CRACKS 1/4" - 1" WIDE

- 1. SURFACE PREPARATION:
 - REMOVE ALL LOOSE, DETERIORATED CONCRETE, DIRT, OIL, GREASE, AND ALL BOND-INHIBITING MATERIALS FROM SURFACE.
 - PROVIDE A MINIMUM REPAIR DEPTH OF $^1\!/_8$ "
 - PREPARATION WORK SHOULD BE DONE BY SCABBLER, CHISELING, WIRE BRUSHING OR OTHER APPROPRIATE MECHANICAL MEANS.
 - ROUGHEN CONTACT SURFACE WITH A MINIMUM PROFILE OF APPROXIMATELY
 - $\frac{1}{16}$ " FOR BONDING WITH NEW MORTAR. - SATURATE SURFACE WITH CLEAN WATER.
 - SUBSTRATE SHOULD BE SATURATED SURFACE DRY WITH NO STANDING WATER DURING APPLICATION.
- 2. APPLICATION AND FINISH:
 - MIX COMPONENTS OF PATCHING MORTAR AND EPOXY ADHESIVE IN
 - ACCORDANCE TO THE MANUFACTURER'S SPECIFICATIONS. - APPLY EPOXY ADHESIVE ONTO THE CONCRETE WITH A BRUSH OR BROOM.
- APPLY THE PATCHING MORTAR WHILE THE EPOXY ADHESIVE IS STILL TACKY. IF THE COATING BECOMES GLOSSY AND LOSES TACKINESS, REMOVE ANY SURFACE CONTAMINANTS AND RECOAT WITH ADDITIONAL ADHESIVE EPOXY
- AND PROCEED WITH PATCHING WORK. - SCRUB REPAIR MORTAR INTO THE SUBSTRATE, FILLING ALL PORES AND
- VOIDS. FORCE MATERIAL AGAINST EDGE OF REPAIR, WORKING TOWARDS THE - MATERIAL MAYBE APPLIED IN MULTIPLE LIFTS. EACH LIFT THICKNESS SHALL
- NOT BE LESS THAN $\frac{1}{8}$ " NOR GREATER THAN 3" THICK. - WHERE MULTIPLE LIFTS ARE REQUIRED, SCORE TOP SURFACE OF EACH LIFT TO PRODUCE A ROUGHENED SURFACE FOR NEXT LIFT. ALLOW PRECEDING

LIFT TO REACH FINAL SET, 30 MINUTES MINIMUM, BEFORE APPLYING FRESH

- MATERIAL.
- SATURATE SURFACE OF THE LIFT WITH CLEAN WATER. - SCRUB FRESH MORTAR INTO PRECEDING LIFT.
- AFTER FILLING REPAIR, CONSOLIDATE, THEN SCREED.
- ALLOW MORTAR TO SET TO DESIRED STIFFNESS, THEN FINISH WITH WOOD OR SPONGE FLOAT FOR A SMOOTH SURFACE.
- 3. CURING:
 - CURING SHOULD COMMENCE IMMEDIATELY AFTER FINISHING. - IF NECESSARY, PROTECT NEWLY APPLIED MATERIAL FROM DIRECT SUNLIGHT,
 - WIND, RAIN OR FROST.
 - MOIST CURE WITH FINE MIST OF WATER OR WITH WET BURLAP AND POLYETHYLENE.
- 4. CRACK REPAIR INCLUDING THE COST OF CEMENTITIOUS MORTAR SHALL BE PAID UNDER THE ITEM "EPOXY INJECTION CRACK REPAIR". SEE SPECIAL PROVISIONS.
- 5. FOR CRACKS OR GAPS IN CONCRETE SURFACE GREATER THAN 1", USE PATCH REPAIR DETAIL.

					DES
-	-	-	_	THE INFORMATION, INCLUDING ESTIMATED	
-	-	-	-	QUANTITIES OF WORK, SHOWN ON THESE SHEETS IS BASED ON LIMITED	CHE
-	-	-	_	INVESTIGATIONS BY THE STATE AND IS	
-	-	-	-	IN NO WAY WARRANTED TO INDICATE THE CONDITIONS OF ACTUAL QUANTITIES	
-	-	-	_	OF WORK WHICH WILL BE REQUIRED.	
_	-	-	_		

SHEET NO. Plotted Date: 8/9/2016

REVISION DESCRIPTION

REV. DATE

MSF BSH

SCALE AS NOTED



Filename: ...\1766 Substructure Conc Crack Repair.dgn

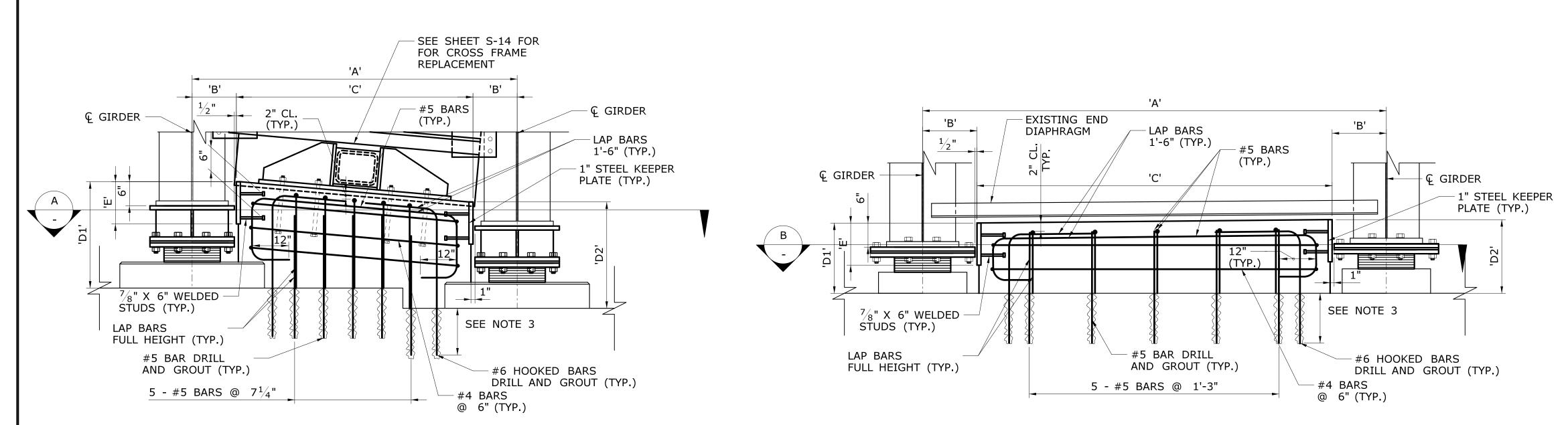


REHABILITATION OF BRIDGE **NO. 01766 I-84 WESTBOUND** OVER AMTRAK AND LOCAL ROADS SUBSTRUCTURE REPAIR -

HARTFORD DRAWING TITLE:

S-10 SHEET NO. 03.04.10 **DETAILS 2**

63-701



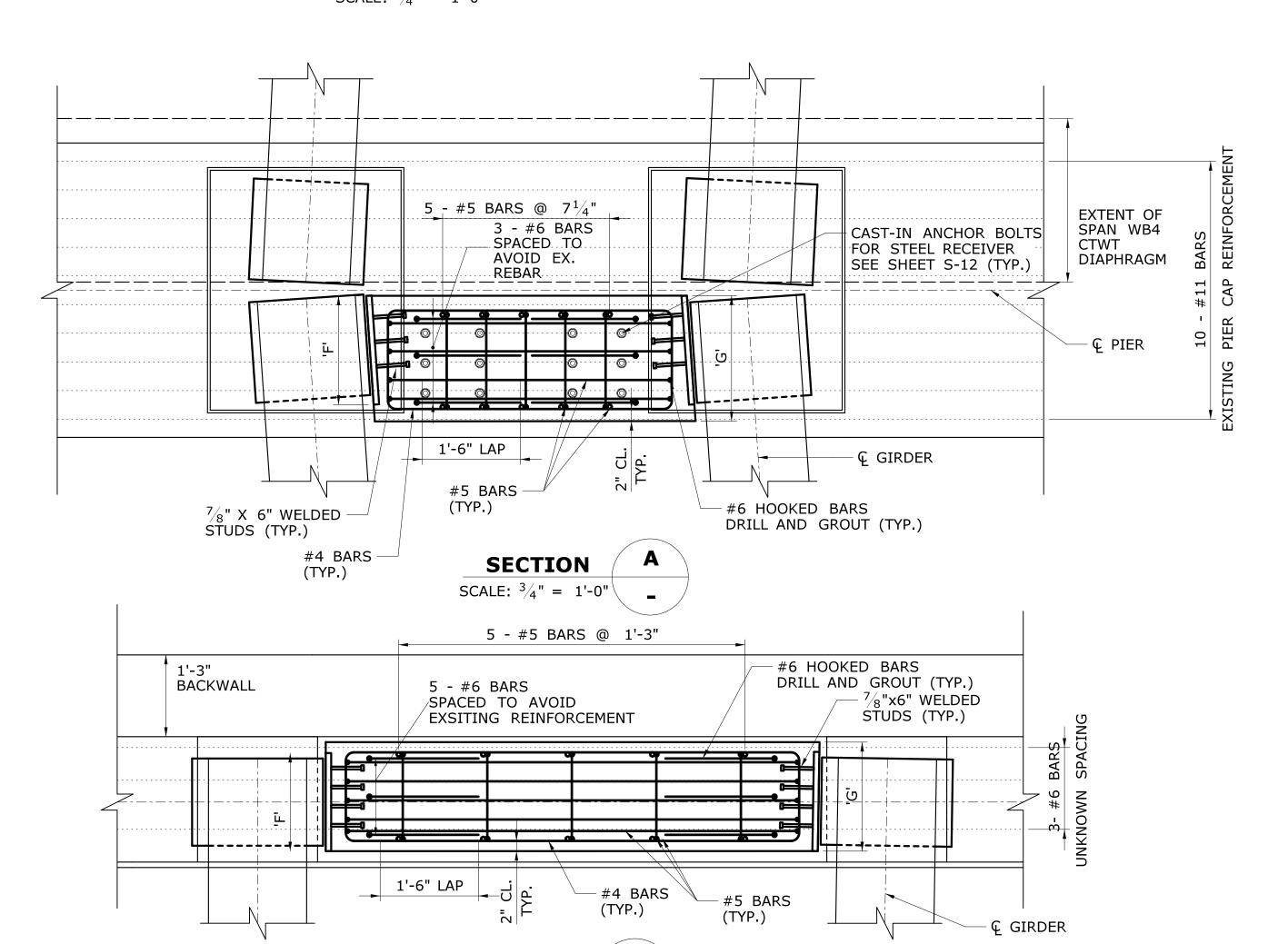
KEEPER BLOCK 'A' (PIER WB3 WEST SHOWN)

SCALE: $\frac{3}{4}$ " = 1'-0"

KEEPER BLOCK 'B' (ABUTMENT 3-N SHOWN)

SCALE: $\frac{3}{4}$ " = 1'-0"

(ABUTMENT 3-N SIMILAR)



SECTION

SCALE: $\frac{3}{4}$ " = 1'-0"

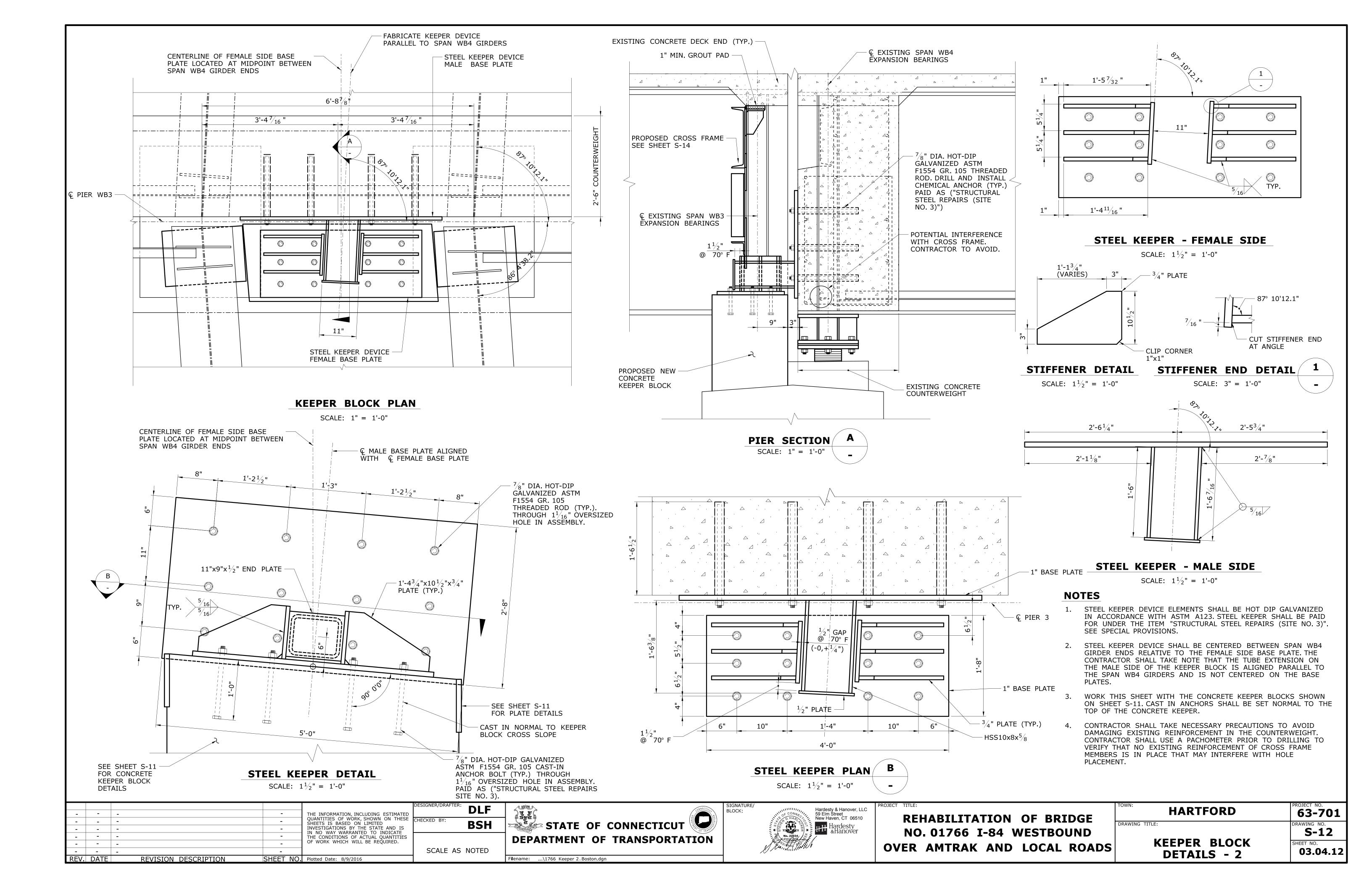
NOTES:

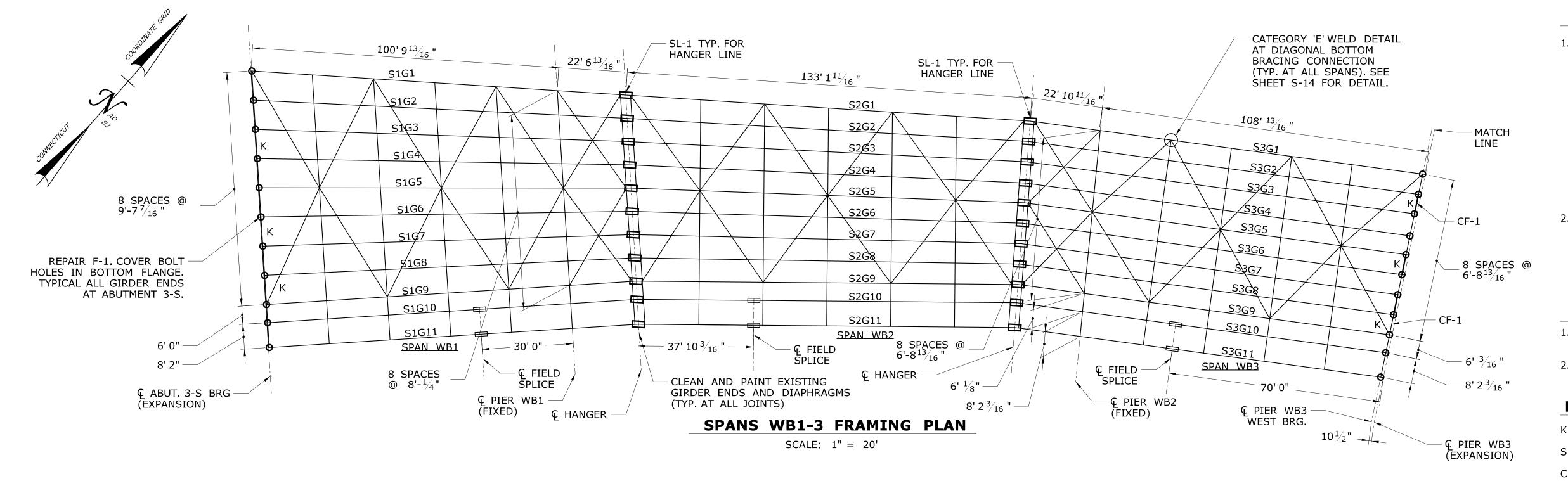
- 1. KEEPER BLOCK CONCRETE SHALL BE PAID FOR UNDER THE ITEM "CLASS 'F' CONCRETE". KEEPER BLOCK REINFORCEMENT SHALL BE PAID FOR UNDER THE ITEMS "DRILLING HOLES AND GROUTING BARS" AND "DEFORMED STEEL DOWELS"
- 2. INSTALL KEEPER BLOCKS AFTER BEARING REPLACEMENT IS COMPLETED.
- DRILLED AND GROUTED DOWELS SHALL BE EMBEDDED SUFFICIENT TO DEVELOP THE YIELD STRENGTH OF THE BAR.
- CONTRACTOR SHALL MEET THE EPOXY ADHESIVE MANUFACTURERS INSTALLATION, SPACING, AND EDGE DISTANCE REQUIREMENTS. IN THE EVENT THAT THE PIER CAP LAYOUT DOES NOT ALLOW FOR THE LAYOUT AND EMBEDMENT SHOWN, THE CONTRACTOR MAY ADJUST REINFORCEMENT WITH APPORVAL OF THE ENGINER.
- CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO AVOID DAMAGING EXISTING REINFORCEMENT. CONTRACTOR SHALL USE A PACHOMETER PRIOR TO DRILLING TO VERIFY THAT NO EXISTING REINFORCEMENT IS IN PLACE THAT MAY INTERFERE WITH HOLE PLACEMENT.
- ROUGHEN THE SURFACE OF THE EXISTING PIER OR ABUTMENT INCLUDING PEDESTAL PRIOR TO PLACEMENT OF NEW CONCRETE.
- 7. STEEL PLATES IN KEEPER BLOCKS SHALL CONFORM TO ASTM A709 STEEL AND SHALL RECEIVE ONE COAT OF SHOP APPLIED PRIMER AND TOP COAT OF PAINT. SURFACES TO BE EMBEDDED IN CONCRETE SHALL RECEIVE PRIME COAT ONLY. STEEL PLATES PAID FOR AS "STRUCTURAL STEEL REPAIRS (SITE NO. 3)".
- 8. EDGES OF KEEPER BLOCKS SHALL BE BEVELED 1"x1".
- 9. PROTECTIVE COMPOUND FOR BRIDGES SHALL BE APPLIED TO ALL EXPOSED SURFACES OF THE KEEPER BLOCKS.
- 10. WORK THIS SHEET WITH THE STEEL KEEPER DEVICE SHOWN ON SHEET

S-12.								
KEEPER BLOCK DETAILS								
LOCATION	Α	В	С	D1*	D2*	Е	F	G
ABUT 3-S BTW G2-G3	9'-7 ⁷ / ₁₆ "	1'-1 ¹ / ₂ "	7'-4 ⁷ / ₁₆ "	1'-6"	1'-81/8"	10 1/2"	1'-6"	1'-8"
ABUT 3-S BTW G6-G7	9'-7 ⁷ / ₁₆ "	1'-1 ¹ / ₂ "	7'-4 ⁷ / ₁₆ "	1'-6"	1'-8"	10 1/2"	1'-6"	1'-8"
ABUT 3-S BTW G8-G9	9'-7 ³ / ₈ "	1'-1 ¹ / ₂ "	7'-4 ³ / ₈ "	1'-6"	1'-61/2"	10 1/2"	1'-6"	1'-8"
PIER WB3 WEST BTW G2-G3	6'-8 ⁷ / ₈ "	11"	4'-10 ⁷ / ₈ "	2'-21/2"	2'-21/2"	10 1/2"	1'-8"	2'-0"
PIER WB3 WEST BTW G5-G6	6'-8 ¹³ / ₁₆ "	11"	4'-10 ¹³ / ₁₆ "	2'-21/2"	2'-21/2"	10 1/2"	1'-8"	2'-0"
PIER WB3 WEST BTW G8-G9	6'-8 ¹³ / ₁₆ "	11"	4'-10 ¹³ / ₁₆ "	2'-21/2"	2'-3 ³ / ₄ "	10 1/2"	1'-8"	2'-0"
ABUT 3-N BTW G3-G4	8'-1/4"	12½"	5'-111/4"	1'-6 ¹ / ₂ "	1'-61/4"	10 1/2"	1'-6"	1'-8"
ABUT 3-N BTW G5-G6	6'-8 ¹³ / ₁₆ "	12½"	4'-7 ¹³ / ₁₆ "	1'-6 ¹ / ₂ "	1'-61/4"	10 1/2"	1'-6"	1'-8"
ABUT 3-N BTW G8-G9	6'-8 ¹³ / ₁₆ "	12 ¹ / ₂ "	4'-7 ¹³ / ₁₆ "	1'-6 ⁷ / ₈ "	1'-61/4"	10 1/2"	1'-6"	1'-8"

* D1 AND D2 ARE BASED ON THE GIRDER BOTTOM ELEVATION PLUS AN ADDITIONAL 6"

OVARITIES OF WORK, SHOWN ON THESE SHEETS IS BASED ON LIMITED SHEETS IS BASED ON LIMITED SHEET IS BASED ON LIMITED SHEET IS BASED ON LIMITED SHEET IS IN NO WAY WARRANTED TO INDICATE SHEET IN NO. 01766 I-84 WESTBOUND OF WORK WHICH WILL BE REQUIRED. THE CONDITIONS OF ACTUAL QUANTITIES OF CONNECTICUT SHEET IN NO. 01766 I-84 WESTBOUND OF WORK WHICH WILL BE REQUIRED. THE CONDITIONS OF ACTUAL QUANTITIES OF WORK, SHOWN ON THESE SHEET IS IN NO. 101766 I-84 WESTBOUND OF WARRANTED TO INDICATE SHEET IN NO. 01766 I-84 WESTBOUND OF WARRANTED TO INDICATE SHEET IN								
OVARITIES OF WORK, SHOWN ON THESE SHEETS IS BASED ON LIMITED SHEETS IS BASED ON LIMITED SHEET IS BASED ON LIMITED SHEET IS BASED ON LIMITED SHEET IS IN NO WAY WARRANTED TO INDICATE SHEET IN NO. 01766 I-84 WESTBOUND OF WORK WHICH WILL BE REQUIRED. THE CONDITIONS OF ACTUAL QUANTITIES OF CONNECTICUT SHEET IN NO. 01766 I-84 WESTBOUND OF WORK WHICH WILL BE REQUIRED. THE CONDITIONS OF ACTUAL QUANTITIES OF WORK, SHOWN ON THESE SHEET IS IN NO. 101766 I-84 WESTBOUND OF WARRANTED TO INDICATE SHEET IN NO. 01766 I-84 WESTBOUND OF WARRANTED TO INDICATE SHEET IN		THE INFORMATION, INCLUDING ESTIMAT	DESIGNER/DRAFTER: MSF	CONNECTICITY IN	SIGNATURE/ BLOCK: Hardesty & Hanover, 59 Fim Street	PROJECT TITLE:	TOWN: HARTFORD	PROJECT NO. 63-701
OVER AMTRAK AND LOCAL ROADS KEEPER BLOCK SHEET NO.	-	QUANTITIES OF WORK, SHOWN ON THES SHEETS IS BASED ON LIMITED INVESTIGATIONS BY THE STATE AND IS IN NO WAY WARRANTED TO INDICATE THE CONDITIONS OF ACTIVAL QUANTITIES	E CHECKED BY: BSH	OF TRANS	** ** ** ** ** ** ** ** ** ** ** ** **			
REVIDATE REVISION DESCRIPTION SHEET NO Plotted Date: 8/10/2016	-	OF WORK WHICH WILL BE REQUIRED.	SCALE AS NOTED	DEPARTMENT OF TRANSPORTATION	J. J	OVER AMTRAK AND LOCAL ROA	DS KEEPER BLOCK DETAILS - 1	SHEET NO. 03.04.11





FIELD PAINTING NOTES

- 1. THE ENDS OF EXISTING GIRDERS AND END DIAPHRAGMS/CROSS FRAMES (IN THEIR ENTIRETY) INCLUDING CONNECTION PLATES, BEARING STIFFENERS, AND SUPPORT BRACKETS SHALL BE CLEANED AND PAINTED IN ACCORDANCE WITH THE SPECIFICATIONS "ABRASIVE BLAST CLEANING AND FIELD PAINTING OF BEAM ENDS (SITE NO. 3)", SEE SPECIAL PROVISIONS. THE CONTAINMENT FOR THE PAINTING SHALL BE PAID UNDER THE ITEM "CLASS 1 CONTAINMENT AND COLLECTION OF SURFACE PREPARATION DEBRIS (SITE NO. 3)", SEE SPECIAL PROVISIONS. DISPOSAL OF LEAD DEBRIS SHALL BE PAID UNDER THE ITEM "DISPOSAL OF LEAD DEBRIS FROM ABRASIVE BLAST CLEANING", SEE SPECIAL PROVISIONS.
- 2. THE 14,050 SQUARE FEET OF ESTIMATED SURFACE AREA OF EXISTING GIRDERS AND END DIAPHRAGMS/CROSS FRAMES TO BE CLEANED & PAINTED IS APPROXIMATE THE CONTRACTOR SHALL SURVEY THE EXISTING BRIDGE STRUCTURE AND REVIEW THE EXISTING PLANS TO FAMILIARIZE HIMSELF WITH THE AREA TO BE CLEANED AND

FRAMING PLAN NOTES:

- 1. PIER DIMENSIONS ARE MEASURED ALONG FASCIA GIRDER G1 EACH SPAN.
- 2. BEAM NUMBERING CONVENTION BASED ON LATEST INSPECTION REPORTS. NUMBERING DIFFERS FROM ORIGINAL CONTRACT DRAWINGS.

LEGEND:

PAINTED.

K - DENOTES APPROX. LOCATION OF CONCRETE KEEPER - 9

SL-# - SEISMIC LOCK RETROFIT - 44

CF-# - CROSS FRAME REPLACEMENT - 2

F-# - STRUCTURE MODIFICATION - 22

- BEARING REPLACEMENT - 44

REFERENCES

- STRUCTURAL STEEL
- NG NOTES AND LIMITS

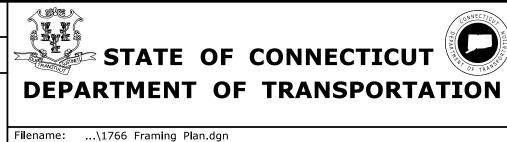
NOTES:

- OF SPAN WB5 DED GIRDER WEBS, ALS CONFORM TO ASTM
- ED IN NOTE 1, WELDED SPLICE MATERIALS

				REFERENCES
	(° ETELD			1. SEE SHEET S-14 TO S-15 FOR ST REPAIRS
	— SL-1 TYP. FOR SPLICE HANGER LINE	© FIELD ————————————————————————————————————		2. SEE SHEET S-33 FOR PAINTING N
	181' ³ / ₈ "	\	$\frac{22'11^{\frac{1}{8}"}}{}$ 3 SPACES @ 6'-11 $\frac{5}{16}$ "	STRUCTURAL STEEL NO
100' 10 ¹ / ₂ " S4G1	S5G1 S5G2	3'-6" (TYP.)	119' 101/4"	I. FOR THE SUSPENDED PORTION O
S4G2 S4G3 S4G4	\$5\hat{G}3\$ \$5\hat{G}3\$ \$5\hat{G}3\$ \$5\hat{G}3\$		\$6G2	2. FOR ALL SPANS NOT INCLUDED IN GIRDER WEBS, FLANGES, AND SPL CONFORM TO ASTM A373.
\$4G5 \$4G6	\$5G6 \$5G7 \$5G8		S6G5	3 SPACES @ 8'-1/4"
8 SPACES @ 54G7 54G8 54G9	S5G9 S5G10		\$6G6 \$6G7 \$6G8	K
S4G10	SPAN WB5	Ç HANGER	\$6G9 \$6G10	5 SPACES @ 6'-8 ¹³ / ₁₆ "
6' ½"	QVER_AMTRAK_ROW 47' 8½"	© FIELD SPLICE	SEAN WB6	K
$60' 7 \frac{3}{16}"$ © PIER WB4 (FIXED)	JI LICE	CATEGORY 'E' WELD DETAIL AT DIAGONAL BOTTOM BRACING CONNECTION (TYP. AT	72' 75/8"	KEIMIN I II COVER BOET
© PIER WB3 (EXPANSION) CLEAN AND PAINT EXISTING GIRDER ENDS AND DIAPHRAGMS (TYP. AT ALL JOINTS)	−Ç HANGER	CONNECTION (TYP. AT ALL SPANS). SEE & PIER WE SHEET S-14 FOR (FIXED DETAIL.	$(E \times 13^{-1})$	HOLES IN BOTTOM FLANGE. TYPICAL ALL GIRDER ENDS AT ABUTMENT 3-N. BRG.
	SPANS WB4-6 FRA	MING PLAN	$-6' \frac{1}{4}" \qquad (EXPAN)$	SION)
	SCALE: 1" =		8' 2 ¹ / ₈ "	

					DES
-	-	-	-	THE INFORMATION, INCLUDING ESTIMATED	
-	-	-	-	QUANTITIES OF WORK, SHOWN ON THESE SHEETS IS BASED ON LIMITED	CHE
-	-	-	-	INVESTIGATIONS BY THE STATE AND IS IN NO WAY WARRANTED TO INDICATE	
-	-	-	-	THE CONDITIONS OF ACTUAL QUANTITIES	
-	-	-	-	OF WORK WHICH WILL BE REQUIRED.	
-	-	-	-		
DE\/	DATE	DEVISION DESCRIPTION	SHEET NO	Plotted Date: 8/9/2016	1

MSF SCALE AS NOTED



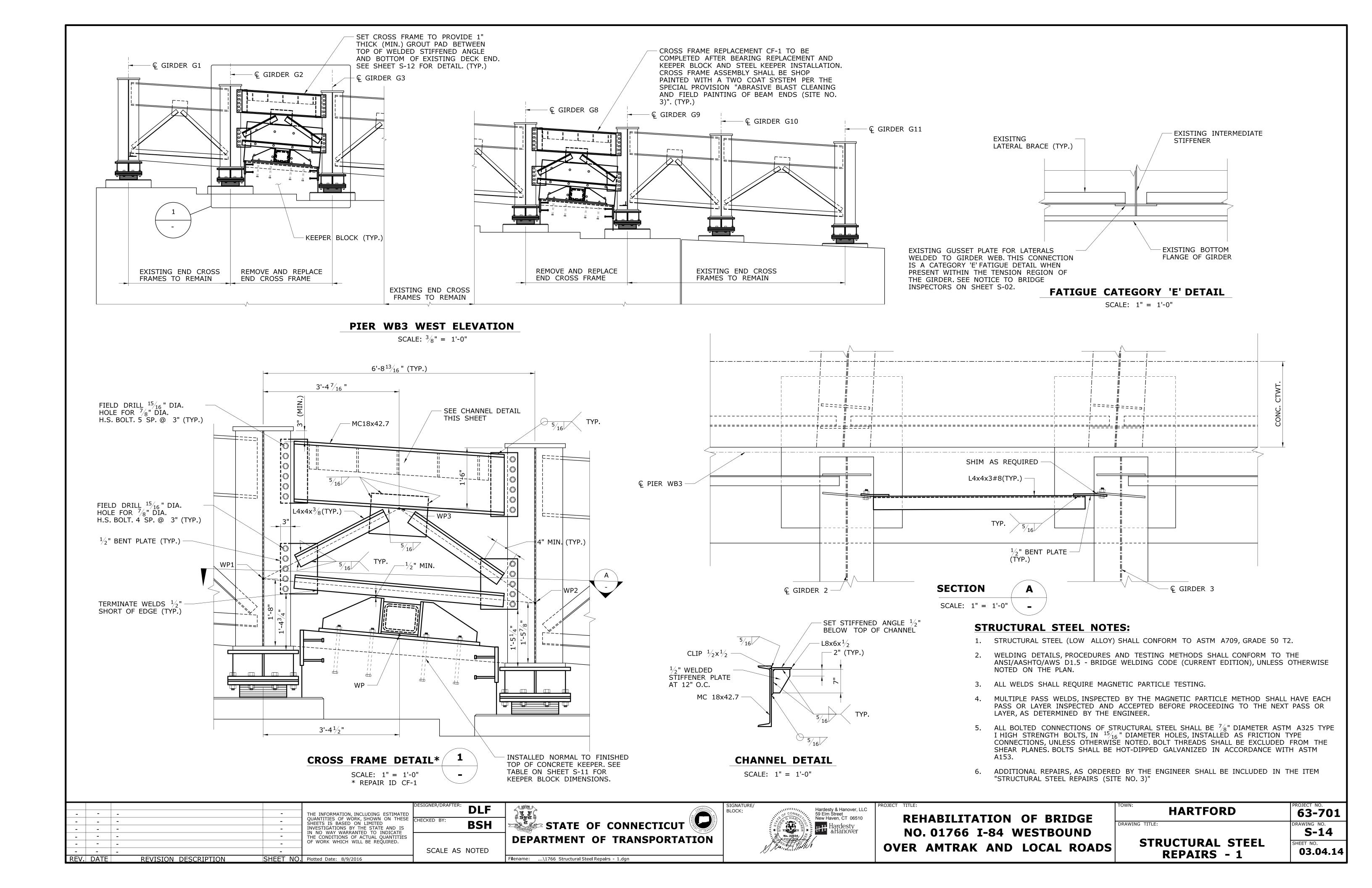


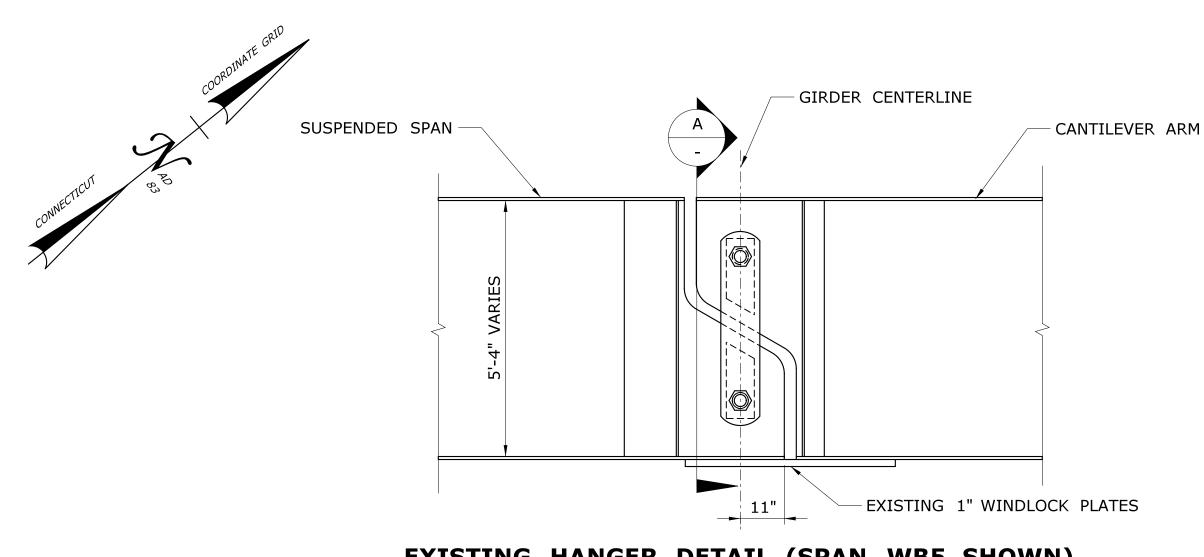
REHABILITATION OF BRIDGE **NO. 01766 I-84 WESTBOUND OVER AMTRAK AND LOCAL ROADS**

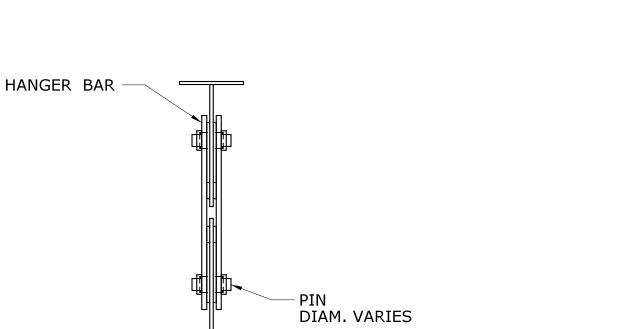
HARTFORD 63-701 DRAWING TITLE:

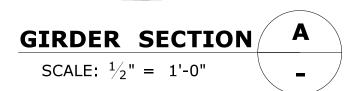
FRAMING PLAN

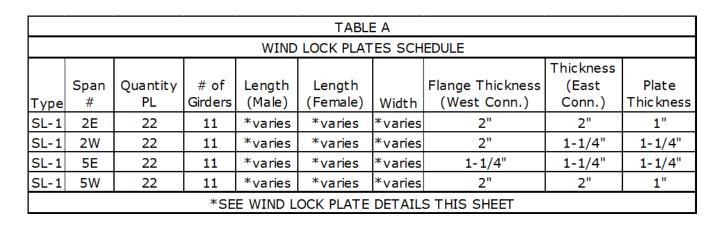
S-13 03.04.13











NOTE: THE SPAN DESIGNATION "W" SHALL BE THE END OF THE SPAN THAT IS DOWNSTATION FROM THE END DESIGNATION "E".

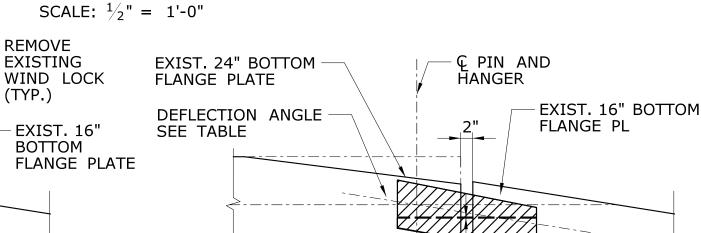
GIRDER FLANGE THICKNESS MAY VARY PER HANGER LINE. FIELD VERIFY.

				TABL	E B	·				
LOCATION				l	DEFLE	CTION	ANGLE			
SPAN	HANGER	G1	G2	G3	G4	G5	G6	G7	G8	G9
9	WEST	9.12	9.28	9.43	9.58	9.73	9.89	10.05	10.21	10.38
9	EAST	10.53	11.15	11.79	12.44	13.09	13.76	14.44	15.13	15.83

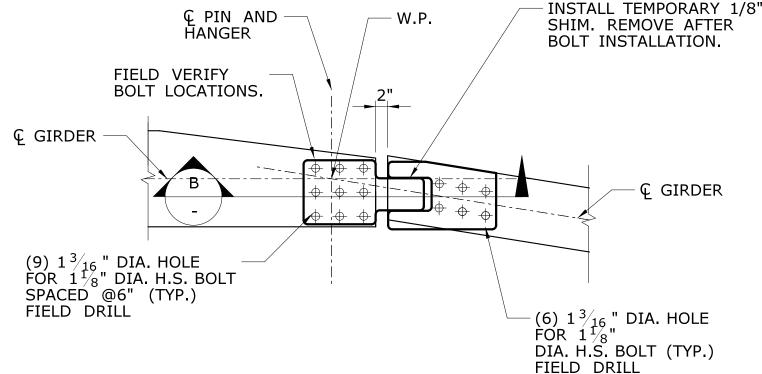


CANTILEVER

SPAN



SUSPENDED



TYPICAL EXISTING WIND LOCK **SPAN WB5 G2 & G6**

C PIN AND

HANGER

EXIST. 24"

FLANGE PLATE

9° 4'44"

SUSPENDED

DEFLECTION ANGLE SEE

TABLE

BOTTOM

SCALE: $\frac{1}{2}$ " = 1'-0"

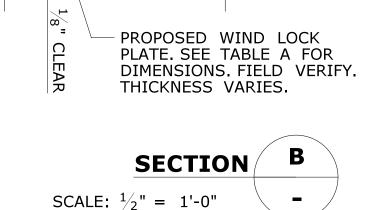
TYPICAL EXISTING WIND LOCK **SPAN WB5 G4 & G8**

- CANTILEVER

SCALE: $\frac{1}{2}$ " = 1'-0"

REMOVE

REPAIR DETAIL SL-1 G1-G11 HANGERS



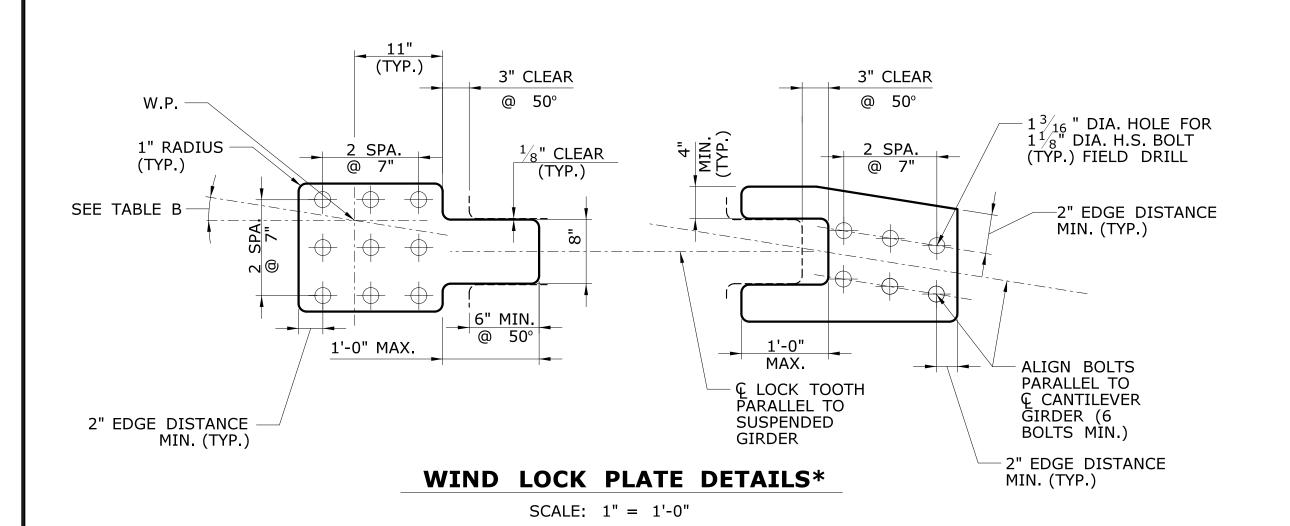
SUSPENDED

NOTES:

- 1. NEW SEISMIC LOCKS SHALL BE INSTALLED AT ALL HANGER GIRDER ENDS. EXISTING SKEWED GIRDER WIND LOCKS SHALL BE REMOVED AND REPLACED AS SHOWN. PROPOSED SEISMIC LOCKS SHOWN SHALL BE PAID FOR AS "STRUCTURAL STEEL REPAIRS (SITE NO. 3)".
- 2. SEISMIC LOCK STEEL PLATES SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123.

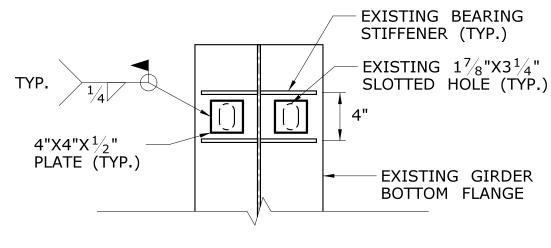
- CANTILEVER

- SIZE AND SHAPE OF PROPOSED SEISMIC LOCK MAY VARY TO SUIT FIELD CONDITIONS. THE CONTRACTOR SHALL DETERMINE ACTUAL CONFIGURATION BASED ON FIELD MEASUREMENTS PRIOR TO FABRICATION. APPROXIMATE DIMENSIONS SHOWN ARE BASED ON THE ORIGINAL CONTRACT PLAN DIMENSIONS.
- STIFFENER SIZE AND LOCATION MAY VARY AT DIFFERENT LOCATIONS. HANGER PLATE DIMENSIONS ARE UNIQUE IN EACH SPAN. FIELD VERIFY BOLT CLEARANCE TO EXISTING
- 5. PROVIDE $\frac{1}{8}$ " CLEARANCE BETWEEN MALE AND FEMALE LOCK ELEMENTS (EACH SIDE). WHERE GIRDERS ARE SKEWED THE GAP SHALL BE ALIGNED WITH THE SUSPENDED
- REMOVAL OF THE EXISTING WIND LOCKS IS INCIDENTAL TO THE ITEM "STRUCTURAL STEEL REPAIRS (SITE NO. 3)".
- 7. LOCALLY CLEAN GIRDER ENDS PER THE SPECIAL PROVISION "ABRASIVE BLAST CLEAN AND FIELD PAINTING OF BEAM ENDS (SITE NO. 3)" PRIOR TO REMOVING EXISTING WIND LOCKS AT ALL GIRDERS AT SPAN EB9 HANGERS (EAST AND WEST). GRIND SMOOTH REMAINING EXISTING WELDS AFTER WINDLOCK REMOVAL
- 8. SEE FRAMING PLAN ON SHEETS S-13 FOR LOCATION OF SEISMIC LOCKS.
- 9. SEE SHEET S-33 FOR FIELD PAINTING REQUIREMENTS.



NOTE: ADJUST BOLT SPACING TO SUIT

EXISTING STIFFENER POSITION



FLANGE HOLE REPAIR F-1 SCALE: 1'' = 1'-0''

(AT EAST AND WEST ABUTMENTS)

BLOCK:

THE INFORMATION, INCLUDING ESTIMATED QUANTITIES OF WORK, SHOWN ON THESE SHEETS IS BASED ON LIMITED - - -INVESTIGATIONS BY THE STATE AND IS - | - | -IN NO WAY WARRANTED TO INDICATE _ | _ _ _ THE CONDITIONS OF ACTUAL QUANTITIES OF WORK WHICH WILL BE REQUIRED. - - - -- - -REV. DATE REVISION DESCRIPTION SHEET NO. Plotted Date: 8/9/2016

NMG BSH

SCALE AS NOTED

STATE OF CONNECTICUT **DEPARTMENT OF TRANSPORTATION**

Filename: ...\1766 Structural Steel Repairs - Seismic Locks.dgn

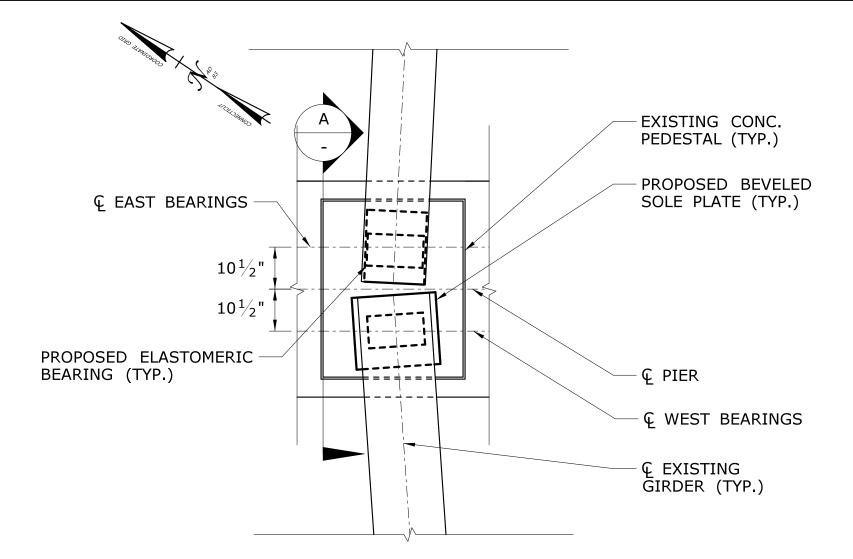


REHABILITATION OF BRIDGE **NO. 01766 I-84 WESTBOUND OVER AMTRAK AND LOCAL ROADS**

HARTFORD 63-701 DRAWING TITLE: S-15 STRUCTURAL STEEL

REPAIRS - 2

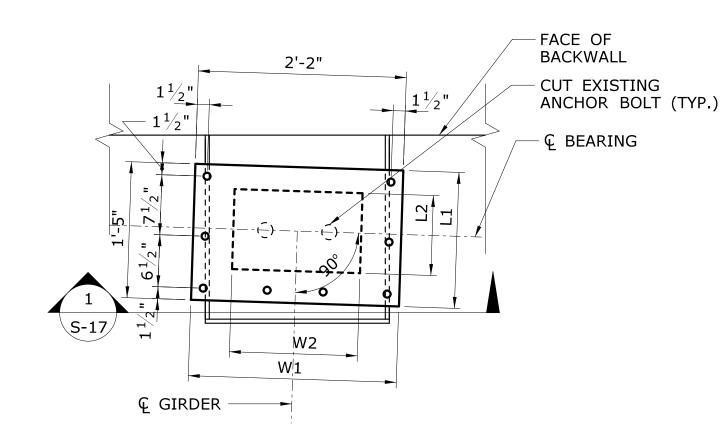
03.04.15

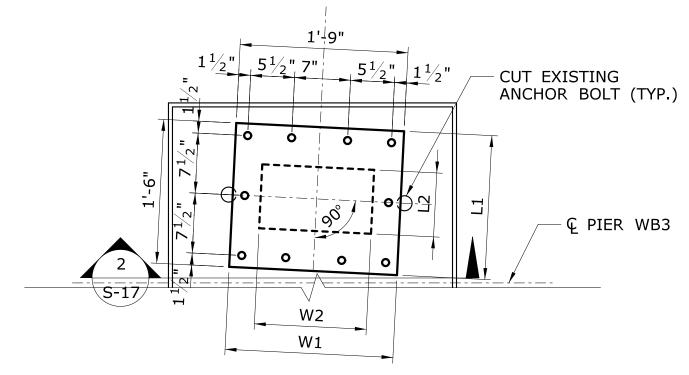


PROPOSED ELASTOMERIC BEARINGS (TYP.) Q EAST BEARINGS PROPOSED 10½" @ 50° F 50° F 50° F

SCALE: $\frac{1}{2}$ " = 1'-0"

PROPOSED ELASTOMERIC BEARING PLAN AT PIER (TYP.) SCALE: $\frac{1}{2}$ " = 1'-0"

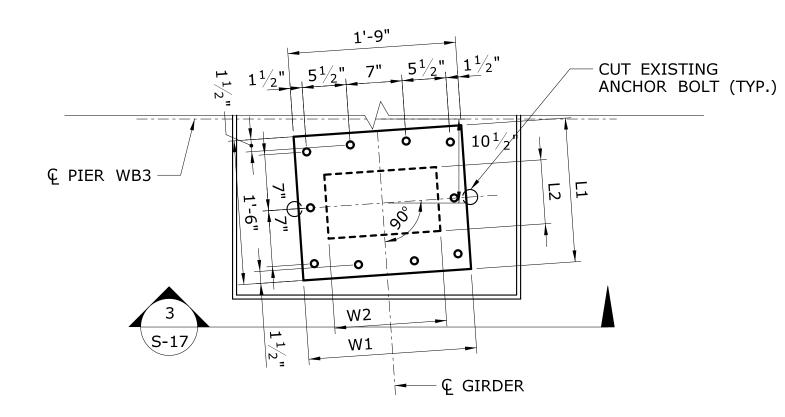


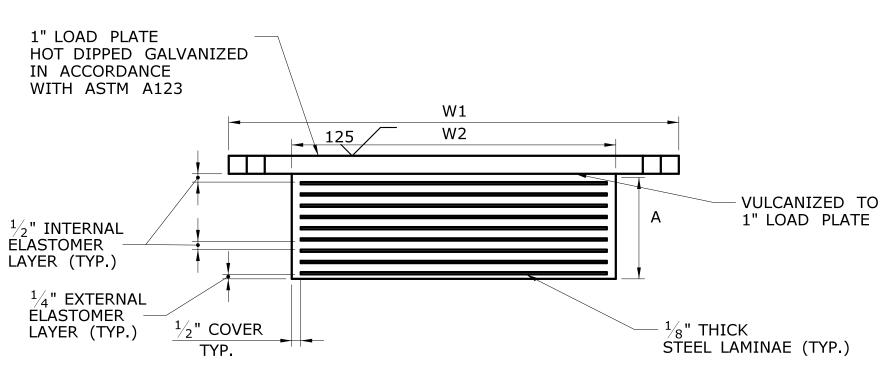


LOAD PLATE PLAN (ABUTMENT 3-S/3-N)

SCALE: 1" = 1'-0"

LOAD PLATE PLAN (PIER WB3 EAST)



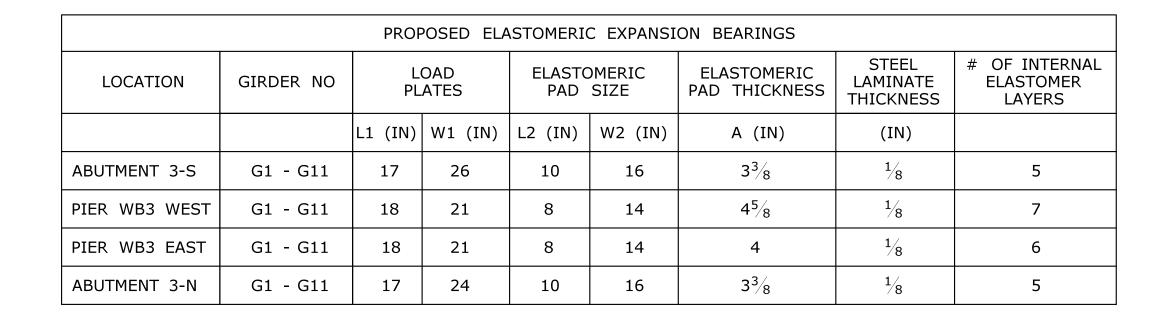


LOAD PLATE PLAN (PIER WB3 WEST)

SCALE: 1" = 1'-0"

ELASTOMERIC SECTION

NOT TO SCALE



INTERIOR ELASTOMERIC BEARING DESIGN LOADS (SERVICE)								
	MAX. DL (KIPS)	MAX. LL + I (KIPS)	MIN. LL + I (KIPS)					
ABUTMENT 3-S	59	110	-29					
PIER WB3 WEST	53	88	-22					
PIER WB3 EAST	48	86	-26					
ABUTMENT 3-N	61	103	-25					

EXTERIOR ELASTOMERIC BEARING DESIGN LOADS (SERVICE)							
	MAX. DL (KIPS)	MAX. LL + I (KIPS)	MIN. LL + I (KIPS)				
ABUTMENT 3-S	52	69	-29				
PIER WB3 WEST	56	62	-22				
PIER WB3 EAST	41	60	-26				
ABUTMENT 3-N	57	70	-25				

NOTES:

- 1. CONTRACTOR TO FIELD VERIFY EXISTING BEARING HEIGHTS PRIOR TO FABRICATION OF EXPANSION BEARINGS TO CONFIRM REQUIRED FILLER PLATE DIMENSIONS.
- 2. FOR PROPOSED CONCRETE KEEPER BLOCK LOCATIONS, SEE SHEETS S-04 TO S-08 AND S-13.
- 3. CLIP SOLE PLATE, BOLSTER, AND LOAD PLATE CORNERS TO AVOID CONFLICT WITH BEAMS FROM THE OPPOSITE DIRECTION. CLEAR DISTANCE FROM PROPOSED BEARING PLATES TO EXISTING FIXED BEARING OR CONCRETE BEARING PAD SHALL BE NO LESS THAN EXISTING CLEAR DISTANCE.
- 4. REINFORCED RECTANGULAR ELASTOMERIC BEARING PADS DESIGNED PER REQUIREMENTS OF AASHTO LRFD SECTION 14, METHOD B.
- 5. ELASTOMERIC BEARING PADS SHALL BE LOW TEMPERATURE GRADE 3, 60 DUROMETER (SHORE A) ELASTOMER, WITH A SHEAR MODULUS OF 0.160KSI AT 73° F.
- 6. STEEL LAMINATE SHALL CONFORM TO ASTM A709, GRADE 36 OR ENGINEER APPROVED EQUIVALENT.
- 7. THE TOP SURFACE OF THE NEW SOLE PLATE SHALL BE BEVELED TO MATCH SLOPE OF THE BOTTOM FLANGE OF GIRDER AFTER THE APPLICATION OF FULL DEAD LOAD. SLOPE TO BE PROVIDED ON SHOP DRAWINGS.
- 8. FURNISHING AND FABRICATING ELASTOMERIC BEARING PAD AND STEEL LOAD PLATE SHALL BE PAID FOR AS ITEM "BEARING REPLACEMENT WITH ELASTOMERIC BEARING PADS".
- 9. ELASTOMERIC BEARING PADS SHALL BE INSTALLED AT AN AMBIENT TEMPERATURE BETWEEN 50° F AND 80° F. JACK & RESET IF PLACED OUTSIDE OF THIS RANGE.
- SEE S-17 FOR STEEL BOLSTER DETAILS AND SUGGESTED BEARING REPLACEMENT PROCEDURE.

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-	-	-	-	THE CONDITIONS OF ACTUAL QUANTITIES	
-	-	-	-	OF WORK WHICH WILL BE REQUIRED.	
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REV.	DATE	REVISION DESCRIPTION	SHEET NO.	Plotted Date: 8/9/2016	



SCALE AS NOTED

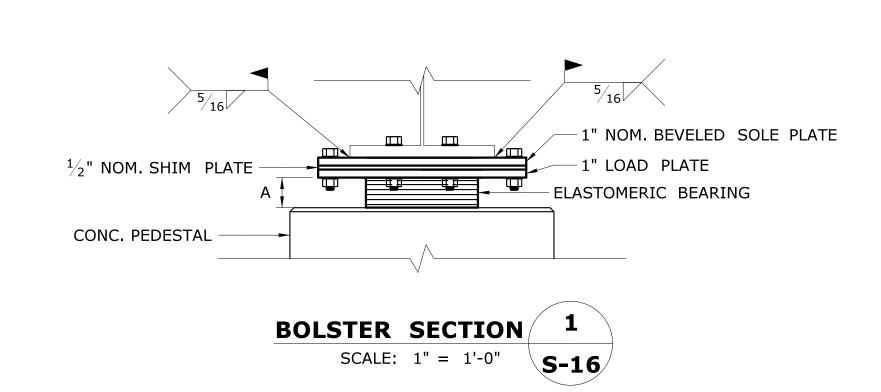


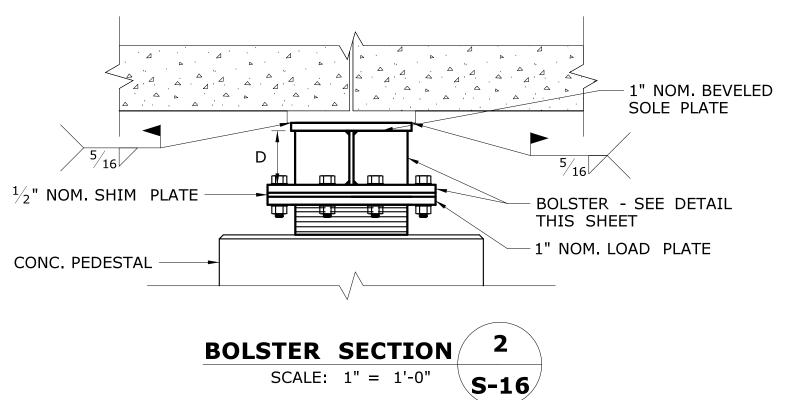
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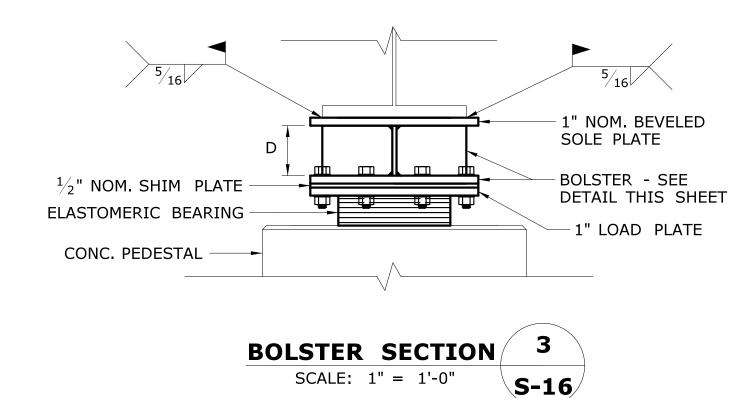


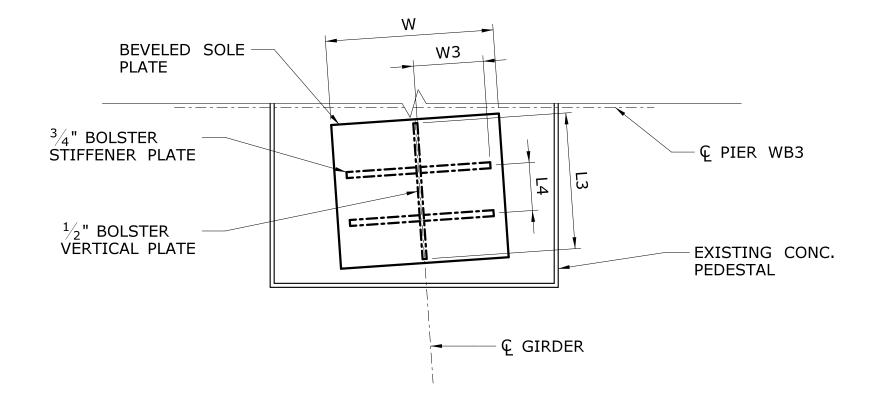
PROJECT	TITLE.
	REHABILITATION OF BRIDGE
	NO. 01766 I-84 WESTBOUND
OV	ER AMTRAK AND LOCAL ROADS

	HARTFORD	PROJECT NO. 63-701
	DRAWING TITLE:	DRAWING NO. S-16
)S	EXPANSION BEARING REPLACEMENT - 1	SHEET NO. 03.04.16





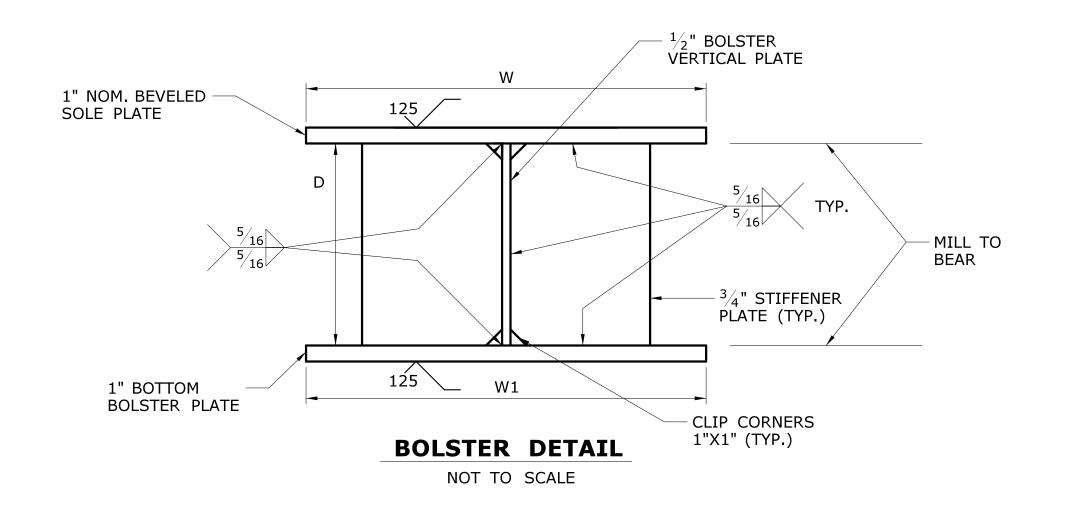




BOLSTER PLAN TYPICAL SCALE: 1" = 1'-0"

	PROPOSED BOLSTERS										
LOCATION	GIRDER NO	BEVELED 1" SOLE PLATE		BOTTOM BOLSTER PLATES		BOLSTER VERTICAL WEB PLATE		BOLSTER VERTICAL STIFFENERS*			
		L (IN)	W (IN)	L1 (IN)	W1 (IN)	L3 (IN)	D (IN)	W3 (IN)	D (IN)	L4 (IN)	
PIER WB3 WEST	G1 - 11	18	21	18	21	17	5 ⁷ / ₈ "	83/4	83/4	6	
PIER WB3 EAST	G1 - 11	18	15	18	21	17	6 ½"	6 ³ / ₄	6 ³ / ₄	6	
ABUT. 3-S	G1 - 11	17	26	17	26						
ABUT. 3-N	G1 - 11	17	24	17	24						

* 4 VERTICAL STIFFENER PLATES SHALL BE USED PER BOLSTER ASSEMBLY



SUGGESTED BEARING REPLACEMENT SEQUENCE

- A. INSTALL JACKING STIFFENERS AS REQUIRED TO SUPPORT JACKING LOADS. SEE SHEET S-18 FOR JACKING REQUIREMENT.
- B. BRACE ROCKER AGAINST ROTATION PRIOR TO JACKING.
- INSTALL JACKS AND RAISE SUPERSTRUCTURE UNTIL LOAD IS REMOVED FROM EXISTING STEEL BEARINGS. ALL BEARINGS ALONG A BEARING LINE TO BE JACKED SIMULTANEOUSLY DURING THE LIFTING OPERATION.
- . REMOVE WELDS BETWEEN BOTTOM FLANGE AND BEARING SOLE PLATE.
- E. REMOVE AND LIFT BEARING ASSEMBLY AND CUT EXISTING ANCHOR BOLTS BELOW THE SURFACE OF PEDESTAL AND GROUT.
- F. PROVIDE A CLEAN LEVEL BEARING SURFACE IN ACCORDANCE WITH THE SPECIAL PROVISION "BEARING REPLACEMENT WITH ELASTOMERIC BEARINGS".
- G. PLACE BOLSTER AND ELASTOMERIC PAD ASSEMBLY SO THAT IT IS CENTERED UNDER CENTERLINE OF BEAM AND CENTERLINE OF BEARING STIFFENER (CENTERED ON PAIR IF MULTIPLE). ADD SHIMS AS NECESSARY AND INSTALL BOLTS BETWEEN BOLSTER AND LOAD PLATE.
- H. LOWER JACK AND TRANSFER LOAD TO THE NEW BEARING PADS.
- I. WELD BEVELED SOLE PLATE TO THE BEAM BOTTOM.

NOTES:

- 1. STEEL BOLSTERS SHALL BE PAID FOR AS ITEM "STRUCTURAL STEEL REPAIRS (SITE NO. 3)".
- 2. STEEL BOLSTERS, BOLSTER PLATES, AND LOAD PLATES SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123.
- 3. EXISTING BEARINGS HAVE LEAD BASED PAINT ADJACENT TO WELDS INTENDED FOR REMOVAL.
- 4. FURNISH EXTERNAL LOAD PLATES SHOP VULCANIZED TO ELASTOMERIC BEARING PADS. LOAD PLATES INCLUDED FOR PAYMENT UNDER THE ITEM "BEARING REPLACEMENT WITH ELASTOMERIC BEARING PADS".
- 5. REMOVAL OF PAINT IN VICINITY OF EXISTING BOTTOM FLANGE FOR THE REMOVAL OF EXISTING BEARING ASSEMBLY AND SOLE PLATE SHALL BE PAID UNDER THE ITEM "ABRASIVE BLAST CLEANING AND FIELD PAINTING OF BEAM ENDS (SITE NO. 3)." SEE SPECIAL PROVISIONS.
- 6. MACHINING OF SOLE PLATE AND BOLSTER PLATE SURFACES SHALL BE PERFORMED AFTER GALVANIZING. MACHINED SURFACE SHALL RECEIVE A PRIME COAT AFTER MACHINING.

-	-	-	_	THE INFORMATION, INCLUDING ESTIMATED
-	-	-	-	QUANTITIES OF WORK, SHOWN ON THESE SHEETS IS BASED ON LIMITED
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-	-	-	-	OF WORK WHICH WILL BE REQUIRED.
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REV.	DATE	REVISION DESCRIPTION	SHEET NO	Plotted Date: 8/9/2016

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SCALE AS NOTED

STATE OF CONNECTICUT DEPARTMENT OF TRANSPORTATION

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REHABILITATION OF BRIDGE NO. 01766 I-84 WESTBOUND OVER AMTRAK AND LOCAL ROADS

GE	HARTF	ORD
-	DRAWING TITLE:	
ID		
	EXPANSION	BEA

63-701

DRAWING NO.

S-17

SHEET NO.

03.04.17

EXPANSION BEARING REPLACEMENT - 2

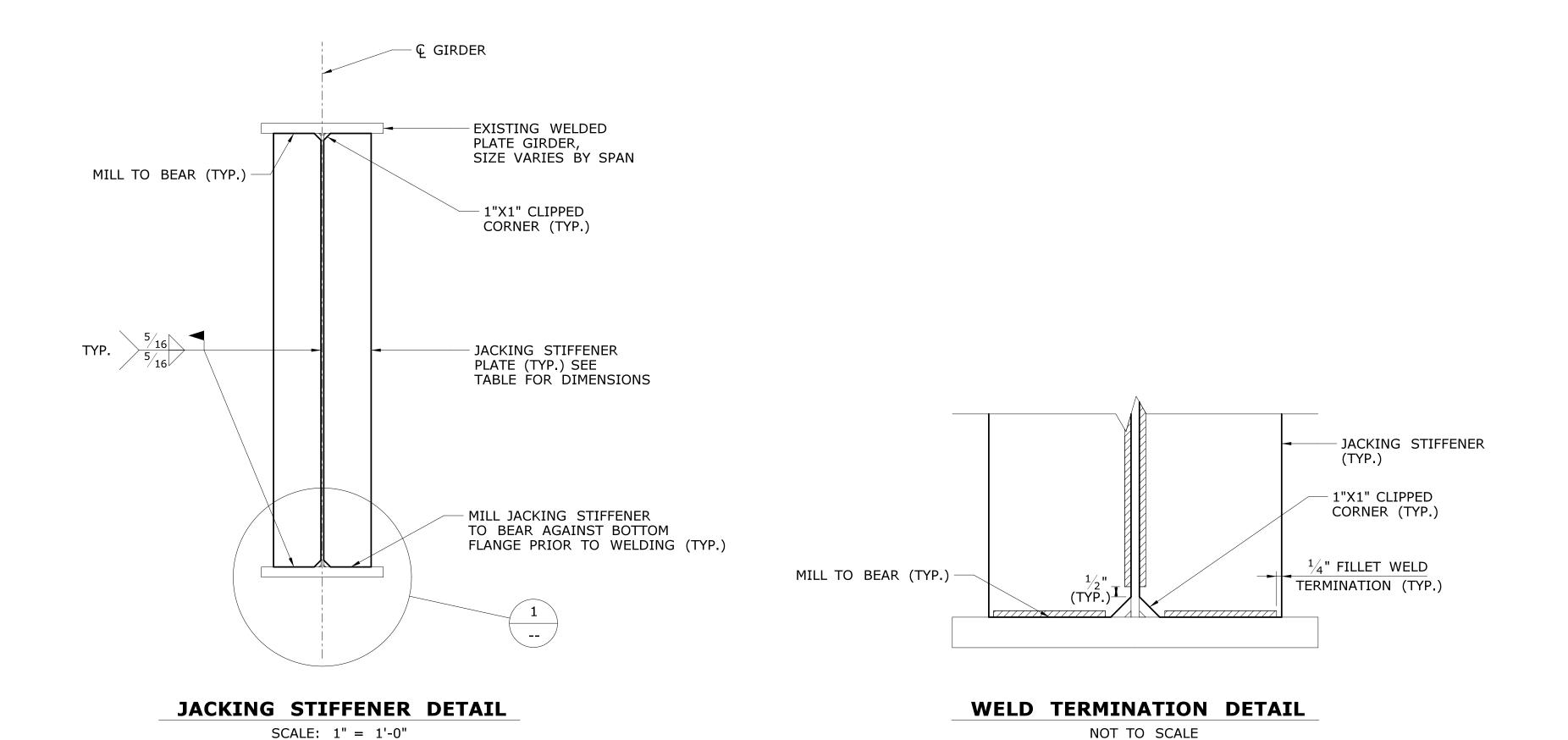
	JACKING LOADS - UNFACTORED *									
			ACKING IGN LOA	NDS	LATERAL LOADS					
LOCATION		DC (KIP)	DW (KIP)	LL + I (KIP)	TRANSVERSE (KIP)	LONGITUDINAL (KIP)				
ABUTMENT	INT	52	8	111	3.1	5.5				
3-S	EXT	44	8	70	3.1	5.5				
PIER WB3	INT	44	9	89	2.8	5.3				
WEST	EXT	48	9	62	2.8	5.3				
PIER WB3	INT	42	6	87	2.7	5.3				
EAST	EXT	35	6	61	2.7	5.3				
ABUTMENT	INT	52	9	104	3.2	5.5				
3-N	EXT	49	9	71	3.2	5.5				

^{*} UNFACTORED JACKING LOADS SHOWN ABOVE ARE TAKEN AT THE EXISTING BEARING LOCATION FOR EACH BEAM. JACKING LOADS SHALL NOT EXCEED 50% OF THE LOAD CAPACITY OF THE JACKS.

	TEMPORARY SPREADER BEAM DESIGN MOMENTS/REACTIONS - UNFACTORED												
		MAX. POSITIVE MOMENTS			MAX. NEGATIVE MOMENTS			NORTH REACTIONS			SOUTH REACTIONS		
LOCATION	GIRDERS	DC (KIP*FT)	DW (KIP*FT)	LL + I (KIP*FT)	DC (KIP*FT)	DW (KIP*FT)	LL + I (KIP*FT)	DC (KIP)	DW (KIP)	LL + I (KIP)	DC (KIP)	DW (KIP)	LL + I (KIP)
PIER WB3 WEST	G1 - G9	2418	495	3757	-18	-4	N/A	207	41	164	193	40	241
PIER WB3 EAST	G1 - G9	2310	330	3757	-14	-2	N/A	187	28	164	185	26	241

NOTICE TO CONTRACTOR:

PRIOR TO ANY CONSTRUCTION, THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL THE SUBSTRUCTURES AND UTILITIES WITHIN THE WORKING AREA. PLACE AND LIMIT CONSTRUCTION EQUIPMENT, CONSTRUCTION LOADS AND OR SURCHARGES IN THE VICINITY OF THE IDENTIFIED SUBSTRUCTURES AND UTILITIES SUCH THAT THE SUBSTRUCTURES AND UTILITIES ARE NOT DAMAGED DUE TO THE CONSTRUCTION ACTIVITIES. MONITOR AND CONTROL VIBRATIONS AND POTENTIAL MOVEMENTS CAUSED BY ANY CONSTRUCTION ACTIVITIES TO AVOID DAMAGES TO THE ADJACENT SUBSTRUCTURES AND UTILITIES. DAMAGES TO ANY SUBSTRUCTURES AND UTILITIES SHALL BE REPAIRED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE CLIENT.



NOTES

- 1. THE PLANS DEPICT A CONCEPTUAL METHOD TO JACK THE BEAMS FOR REPLACING ALL EXPANSION BEARINGS. THE CONTRACTOR MAY SUBMIT ALTERNATE METHODS AND PROCEDURES TO THE ENGINEER FOR REVIEW AND APPROVAL.
- 2. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF ALL TEMPORARY SUPPORT ELEMENTS AND ANY TEMPORARY STRUCTURES REQUIRED TO ACCESS AND PERFORM THE WORK. ALL WORK ASSOCIATED WITH SUPPORT STRUCTURES SHALL BE PAID FOR AS "TEMPORARY SUPPORT ASSEMBLY". THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS AND COMPUTATIONS PREPARED, SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE STATE OF CONNECTICUT, TO THE ENGINEER FOR REVIEW AND APPROVAL.
- 3. THE WORK TO DESIGN THE TEMPORARY JACKING SYSTEM, DEVELOP THE JACKING CONSTRUCTION PROCEDURE, FURNISH AND INSTALL THE NECESSARY HYDRAULIC LIFTING COMPONENTS AND PERFORM THE HYDRAULIC LIFTING OPERATION SHALL BE PAID FOR UNDER THE ITEM "JACKING FOR BEARING REPLACEMENT". SEE SPECIAL PROVISIONS.
- 4. THE DESIGN OF SUPPLEMENTAL STRUCTURAL ELEMENTS TO STRENGTHEN EIXSTING MEMBERS PRIOR TO HYDRAULIC LIFTING IS INCIDENTAL TO THE ITEM "JACKING FOR BEARING REPLACEMENT".
- 5. THE DESIGN, FURNISHING, INSTALLATION AND REMOVAL OF OSHA COMPLIANT WORK PLATFORM AND RAILING SHALL BE PAID FOR AS "JACKING FOR BEARING REPLACEMENT".
- 6. JACKING OPERATIONS SHALL BE PERFORMED UNDER LIVE TRAFFIC. THE CONTRACTOR SHALL DESIGN THE JACKING SUPPORT STRUCTURE FOR THE SPECIFIED BEAM END REACTIONS TABULATED ON THIS SHEET. THE CONTRACTOR MUST ENSURE THAT TRAVEL LANES ARE OPEN TO TRAFFIC IN ACCORDANCE WITH THE SPECIAL PROVISIONS "PROSECUTION AND PROGRESS".
- 7. BEARINGS SHALL BE REMOVED AND REPLACED ONE FOR ONE ON A SINGLE SUBSTRUCTURE UNIT AT A TIME. ALL BEAMS ALONG A SINGLE SUBSTRUCTURE UNIT SHALL BE JACKED SIMULTANEOUSLY DURING LIFTING OPERATIONS.
- 8. VERTICAL JACKING DIFFERENTIAL BETWEEN ADJACENT SPANS SHALL BE LIMITED TO $\frac{1}{2}$ " FOR PIER WB3.
- WHERE EXISTING DOWNSPOUTS AND LEADERS INTERFERE WITH THE JACKING OR SUPPORT ELEMENTS THEY SHALL BE REMOVED AND REPLACED. PAY FOR UNDER THE ITEMS "REMOVE EXISTING BRIDGE DRAINAGE SYSTEM" AND "8" PIPE FOR BRIDGE DRAINAGE" (FIBERGLASS).
- 10. WHERE EXISTING HABITATION EXISTS ON STATE PROPERTY, IT SHALL BE REMOVED AT THE RESIDENT'S DIRECTION. REMOVAL AND DISPOSAL SHALL BE INCLUDED FOR PAYMENT UNDER THE ITEM "CLEARING AND GRUBBING".
- 11. THE USE OF A TEMPORARY SPREAD FOOTING IS PERMITTED AS AN ALTERNATE MEANS OF SUPPORT FOUNDATION WHERE FEASIBLE. SEE NOTES ON SHEET S-19 FOR REQUIREMENTS. THE DESIGN AND INSTALLATION OF SPREAD FOOTING SHALL BE INCLUDED FOR PAYMENT UNDER ITEM "TEMPORARY SUPPORT ASSEMBLY".
- 12. SEE SHEET S-17 FOR SUGGESTED EXPANSION BEARING REPLACEMENT PROCEDURE.
- 13. WORK THE LOAD TABLES ON THIS SHEET WITH THE SUGGESTED TEMPORARY SUPPORT ASSEMBLIES SHOWN ON SHEET S-19.

PROPOSI	ED JACKING STIFFENE	R DIMENSIONS
PIER	WIDTH (IN.)	THICKNESS (IN.)
ABUT. 3-S	8	3/4"
PIER WB3 WEST	8	3/4"
PIER WB3 EAST	7	3/4"
ABUT. 3-N	7	3/4"

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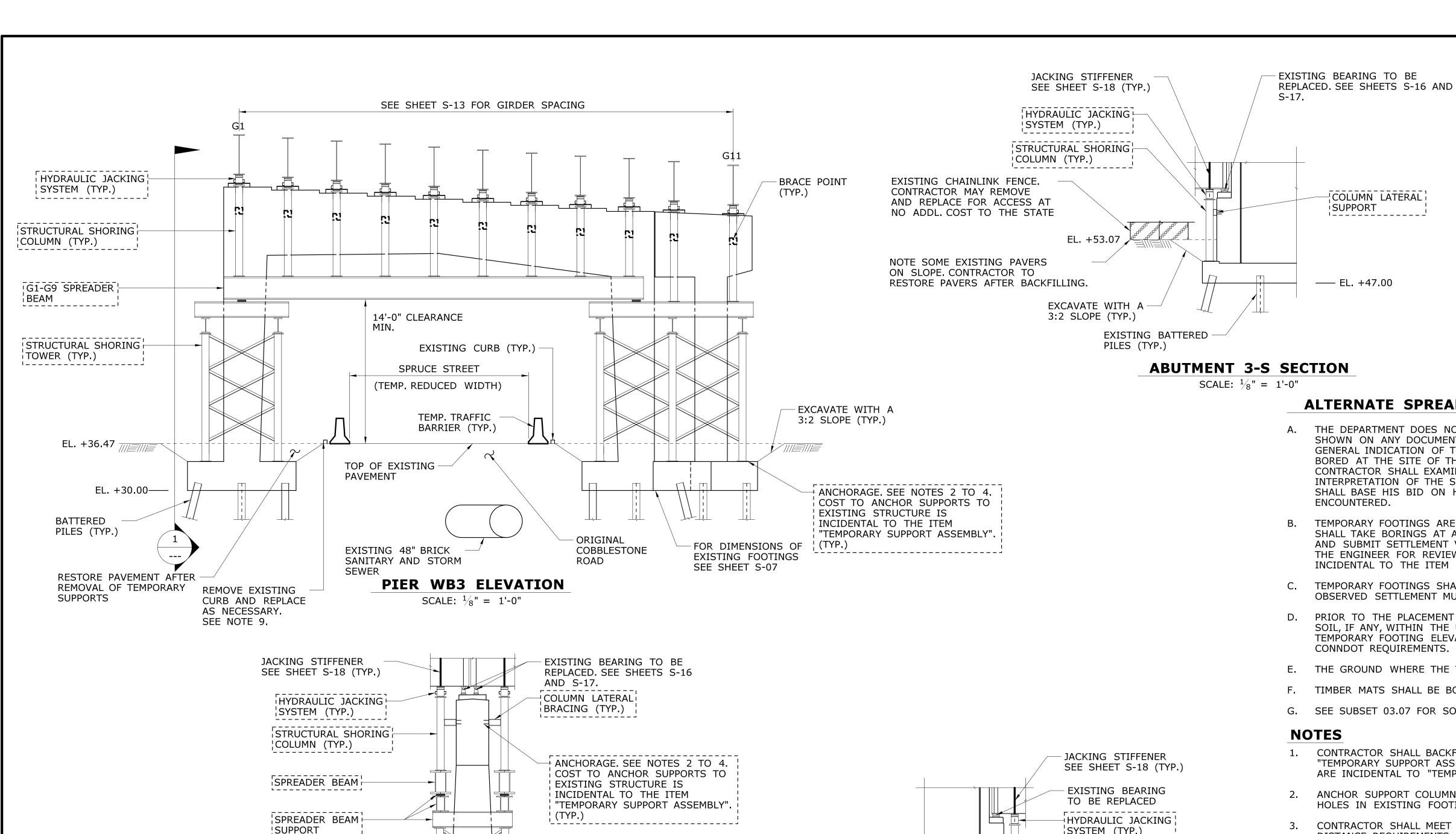


- 1	TITLE.	Ι'`
	REHABILITATION OF BRIDGE	D
	NO. 01766 I-84 WESTBOUND	
V	ER AMTRAK AND LOCAL ROADS	

HARTFORD	PROJECT NO. 63-70
DRAWING TITLE:	DRAWING NO. S-18
TEMPORARY SUPPORT	SHEET NO.

OF STRUCTURE - 1

03.04.18



EXCAVATE WITH A

3:2 SLOPE (TYP.)

EXISTING BATTERED

PILES (TYP.)

STRUCTURAL SHORING

EL. +36.47

PIER WB3 SECTION

SCALE: $\frac{1}{8}$ " = 1'-0"

TOWER (TYP.)

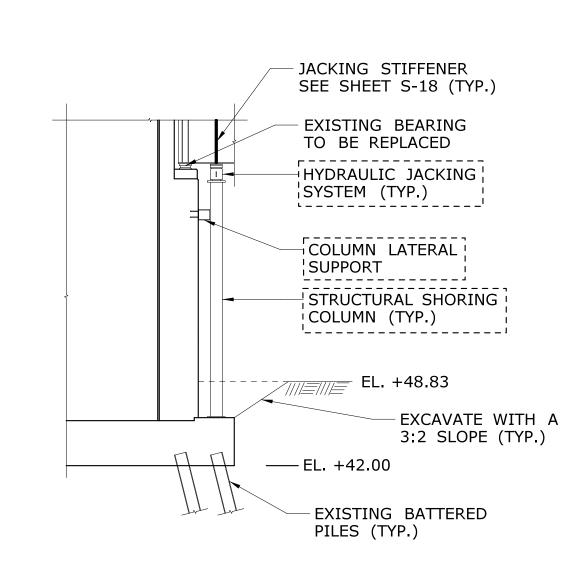
EXCAVATE WITH A

3:2 SLOPE (TYP.)

EXISTING 48" BRICK

SEWER

SANITARY AND STORM



ABUTMENT 3-N SECTION

SCALE: $\frac{1}{8}$ " = 1'-0"

ALTERNATE SPREAD FOOTING NOTES

-COLUMN LATERAL

SUPPORT

—— EL. +47.00

- THE DEPARTMENT DOES NOT GUARANTEE THE DETAILS PERTAINING TO BORINGS, AS SHOWN ON ANY DOCUMENTS SUPPLIED BY THE DEPARTMENT, TO BE MORE THAN A GENERAL INDICATION OF THE MATERIALS LIKELY TO BE FOUND ADJACENT TO HOLES BORED AT THE SITE OF THE WORK, APPROXIMATELY AT THE LOCATIONS INDICATED. CONTRACTOR SHALL EXAMINE BORING DATA, WHERE AVAILABLE, AND MAKE THEIR OWN INTERPRETATION OF THE SUBSOIL INVESTIGATIONS AND OTHER PRELIMINARY DATA AND SHALL BASE HIS BID ON HIS OWN OPINION OF THE CONDITIONS LIKELY TO BE ENCOUNTERED.
- TEMPORARY FOOTINGS ARE ANTICIPATED TO SETTLE ON LOADING. THE CONTRACTOR SHALL TAKE BORINGS AT ALL LOCATIONS WHERE A SPREAD FOOTING IS TO BE USED AND SUBMIT SETTLEMENT VALUES. ALL DESIGN CALCULATIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW. THE COST OF BORING AND LABORATORY SOIL TEST IS INCIDENTAL TO THE ITEM "TEMPORARY SUPPORT ASSEMBLY".
- TEMPORARY FOOTINGS SHALL BE CONTINUOUSLY MONITORED FOR SETTLEMENT AND OBSERVED SETTLEMENT MUST BE COMPENSATED BY JACK ADJUSTMENTS.
- PRIOR TO THE PLACEMENT OF THE TEMPORARY FOOTING, OVER EXCAVATE COHESIVE SOIL, IF ANY, WITHIN THE UPPER FIVE FEET FROM THE PROPOSED BOTTOM OF THE TEMPORARY FOOTING ELEVATION AND REPLACEMENT WITH ENGINEERED FILL AS PER CONNDOT REQUIREMENTS.
- THE GROUND WHERE THE TEMPORARY FOOTING IS SEATED SHALL BE LEVEL.
- TIMBER MATS SHALL BE BOLTED TOGETHER.
- G. SEE SUBSET 03.07 FOR SOIL BORING REFERENCE DATA.

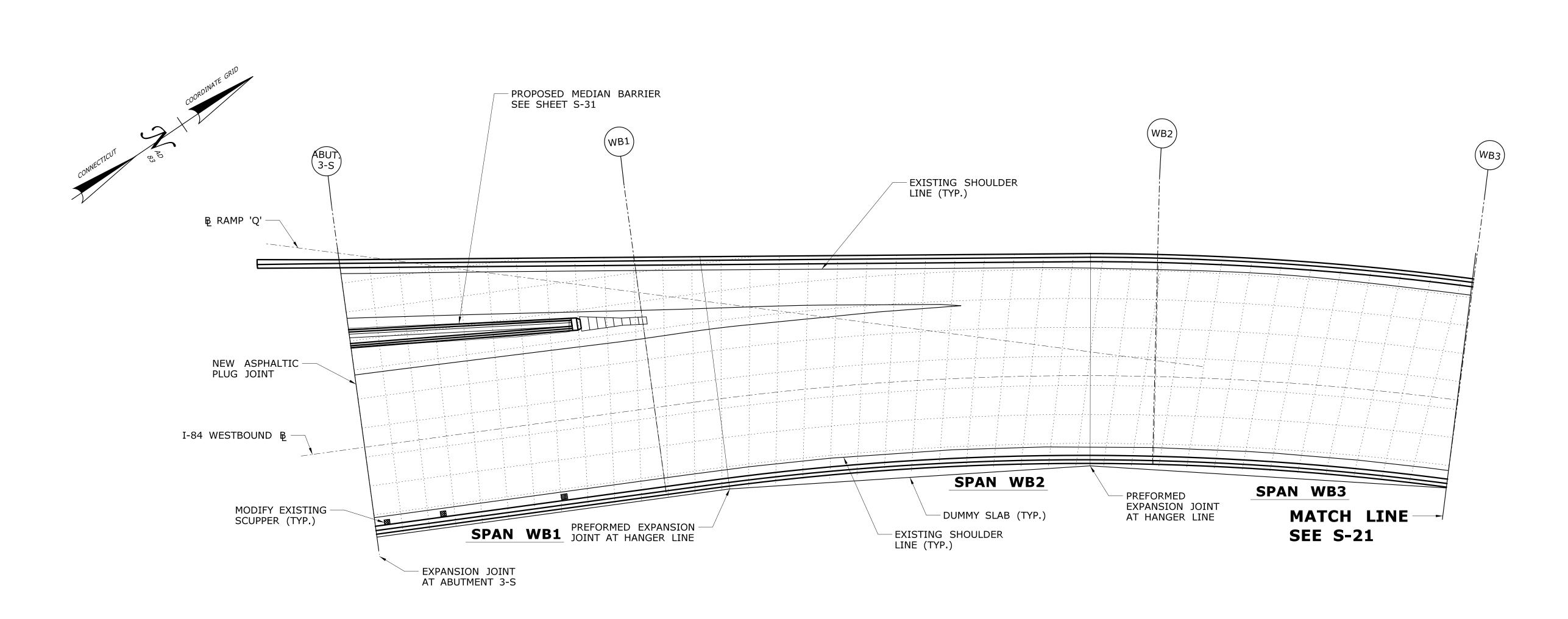
NOTES

- CONTRACTOR SHALL BACKFILL ANY EXCAVATION, COST IS INCIDENTAL TO THE ITEM "TEMPORARY SUPPORT ASSEMBLY", COST OF EXCAVATION AND SUPPORT OF EXCAVATION ARE INCIDENTAL TO "TEMPORARY SUPPORT ASSEMBLY"
- 2. ANCHOR SUPPORT COLUMNS TO EXISTING FOOTING WITH DRILLED AND GROUTED BARS. HOLES IN EXISTING FOOTING SHALL BE CORE DRILLED.
- 3. CONTRACTOR SHALL MEET THE MANUFACTURER'S INSTALLATION, SPACING, AND EDGE DISTANCE REQUIREMENTS FOR ANY DRILLED AND GROUTED BAR. REMOVE ALL ELEMENTS UPON COMPLETION OF THE WORK, PRIOR TO BACKFILLING.
- 4. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO AVOID DAMAGING EXISTING REINFORCEMENT. CONTRACTOR SHALL USE A PACHOMETER PRIOR TO DRILLING TO VERIFY THAT NO EXISTING REINFORCEMENT IS IN PLACE THAT MAY INTERFERE WITH HOLE PLACEMENT.
- EXISTING UTILITIES SHOWN ARE REPRESENTATIVE ONLY. THE CONTRACTOR SHOULD BE AWARE THAT THERE ARE EXISTING UTILITIES IN THE VACINITY OF PROPOSED JACKING LOCATIONS, SURCHARGE ON PIERS DURING CONSTRUCTION SHALL BE APPROVED BY UTILITY OWNER. SEE GENERAL NOTES ON SHEET S-03 FOR EXISTING UTILITY NOTES.
- 6. SEE SHEET S-18 FOR JACKING ASSEMBLY AND LIFTING OPERATION NOTES, AND LOAD TABLES.
- 7. SEE SHEET S-17 FOR SUGGESTED BEARING REPLACMENT PROCEDURE.
- 8. SEE SHEET S-11 AND S-12 FOR KEEPER BLOCKS.
- REMOVAL AND REPLACEMENT OF CURB SHALL BE PAID FOR AS "RESET CONCRETE CURBING".

LEGEND

DENOTES CONTRACTOR DESIGNED ELEMENTS

		DESIGNER/DRAFTER:	40 SSN-028 D.	SIGNATURE/	PROJECT TITLE:	TOWN:	PROJECT NO.
	THE INFORMATION, INCLUDING ESTIMATED	DESIGNER/DRAFTER: MSF	- Santa Sant	BLOCK: Hardesty & Hanover, LLC 59 Elm Street New Haven, CT 06510	REHABILITATION OF BRIDGE	HARTFORD	63-701
	SHEETS IS BASED ON LIMITED INVESTIGATIONS BY THE STATE AND IS	BSH	STATE OF CONNECTICUT	Hardesty & Hanover	NO. 01766 I-84 WESTBOUND	DRAWING TITLE:	DRAWING NO. S-19
	THE CONDITIONS OF ACTUAL QUANTITIES OF WORK WHICH WILL BE REQUIRED.		DEPARTMENT OF TRANSPORTATION	No. 22034 5 7		TEMPORARY SUPPORT	SHEET NO.
REVISION DESCRIPTION S	- HEET NO Plotted Date: 8/9/2016	SCALE AS NOTED	Filename: \MSta Design\1766 lacking don	M King College	OVER AMTRAK AND LOCAL ROADS	OF STRUCTURE - 2	03.04.19



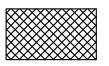
SPANS WB1-3 DECK PLAN

SCALE: 1" = 20'

SPAN*	ANTICIPATED TOTAL PATCH AREA (SF)**	AS-BUILT TOTAL PATCH AREA (SF)
WB1	2710	
WB2	2430	
WB3	2330	

- * "SPAN" IS MEASURED FROM JOINT TO JOINT
- ** SEE NOTE 3

LEGEND:



MODIFY SCUPPER EXTENSION SEE SHEET S-36

NOTES:

- 1. GRID LINES SHOWN ARE APPROXIMATELY 10' x 10'
- 2. FOR RESIDENT USE TRACKING AS-BUILT PATCH LIMITS.
- 3. SEE SHEET S-24 FOR DECK PATCHING DETAILS AND REQUIREMENTS.
- 4. SEE SHEET S-21 FOR SPANS WB4-6.
- 5. SEE SHEET S-22 + S-23 FOR UNDERSIDE DECK PATCHING PLAN.

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-	-	-	-	INVESTIGATIONS BY THE STATE AND IS IN NO WAY WARRANTED TO INDICATE	
-	-	-	-	THE CONDITIONS OF ACTUAL QUANTITIES	
-	-	-	-	OF WORK WHICH WILL BE REQUIRED.	
-	-	-	-		
RFV.	DATE	REVISION DESCRIPTION	SHEET NO	Plotted Date: 8/9/2016	

SCALE AS NOTED

STATE OF CONNECTICUT
DEPARTMENT OF TRANSPORTATION

Filename: ...\1766 Deck Patching Plans.dgn



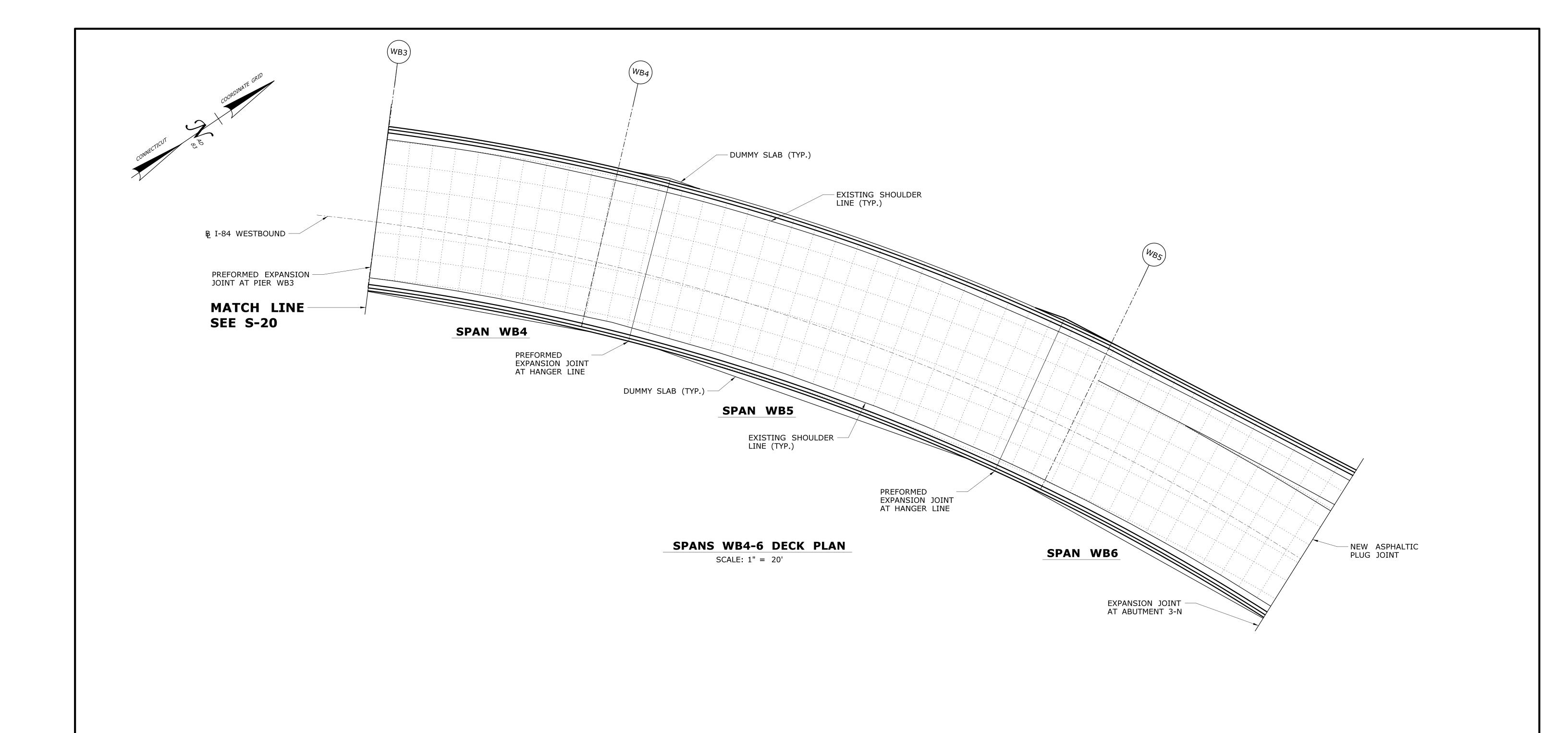
REHABILITATION OF BRIDGE NO. 01766 I-84 WESTBOUND OVER AMTRAK AND LOCAL ROADS

TOWN:	НА	RTFORD	
DRAWING	TITLE:		
	DECV	DATCHING	

DECK PATCHING SHEET NO.
PLAN - 1

DRAWING NO.
S-20
SHEET NO.
03.04.20

PROJECT NO. **63-701**



SPAN*	ANTICIPATED TOTAL PATCH AREA (SF)**	AS-BUILT TOTAL PATCH AREA (SF)
WB4	2620	
WB5	3210	
WB6	2550	

* "SPAN" IS MEASURED FROM JOINT TO JOINT

NOTES:

- 1. GRID LINES SHOWN ARE APPROXIMATELY 10' x 10'
- 2. FOR RESIDENT USE TRACKING AS-BUILT PATCH LIMITS.
- 3. SEE SHEET S-24 FOR DECK PATCHING DETAILS AND REQUIREMENTS.
- 4. SEE SHEET S-20 FOR SPANS WB1-3.
- 5. SEE SHEET S-22 AND S-23 FOR UNDERSIDE.

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-	-	-	-	THE INFORMATION, INCLUDING ESTIMATED
-	-	-	-	QUANTITIES OF WORK, SHOWN ON THESE SHEETS IS BASED ON LIMITED
-	-	-	-	INVESTIGATIONS BY THE STATE AND IS IN NO WAY WARRANTED TO INDICATE
-	-	-	-	THE CONDITIONS OF ACTUAL QUANTITIES
-	-	-	-	OF WORK WHICH WILL BE REQUIRED.
-	-	-	-	
REV.	DATE	REVISION DESCRIPTION	SHEET NO.	Plotted Date: 8/9/2016

ESIGNER/DRAFTER:	MSF	
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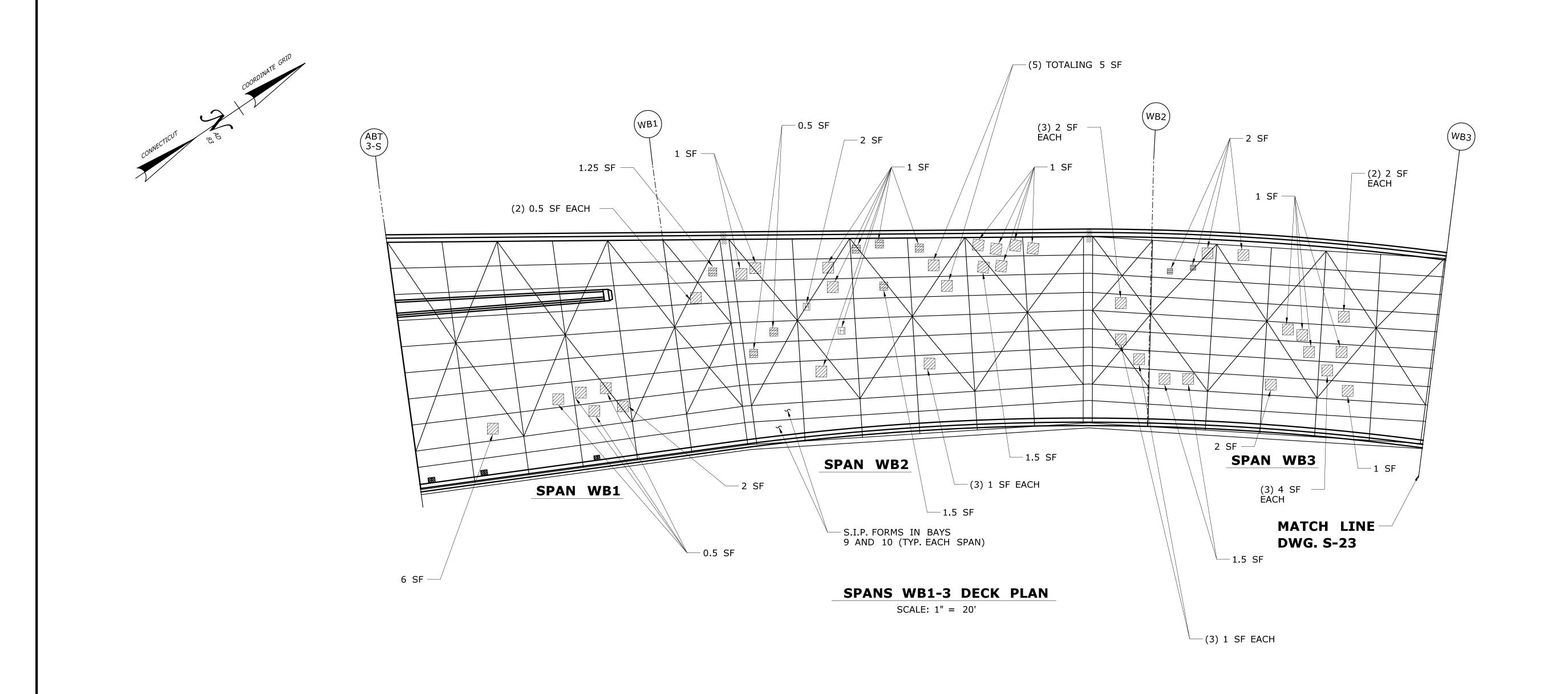
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	HARTFORD	PROJECT NO. 63-701
	DRAWING TITLE:	S-21
S	DECK PATCHING PLAN - 2	03.04.21

^{**} SEE NOTE 3



CONCRETE DETERIORATION LEGEND:

HOLLOW HAUNCH

SPALL

SPALL WITH EXPOSED REINFORCEMENT

S.F. SQUARE FEET

S.I.P FORM RUSTED THROUGH

HALLOW AREA

FL HOLLOW HAUNCH FOR FULL DIAPHRAGM BAY LENGTH

NOTES:

- THE DETERIORATION SHOWN IS BASED ON INSPECTION INFORMATION.
 THE CONTRACTOR AND RESIDENT ENGINEER ARE RESPONSIBLE FOR FINAL
 LIMITS.
- 2) WORK THIS SHEET WITH UNDERSIDE PATCHING SHEET S-23 AND ESTIMATED QUANTITIES SHOWN ON SHEETS S-20 AND S-21.
- 3) SEE S-24 FOR DECK PATCHING DETAILS.

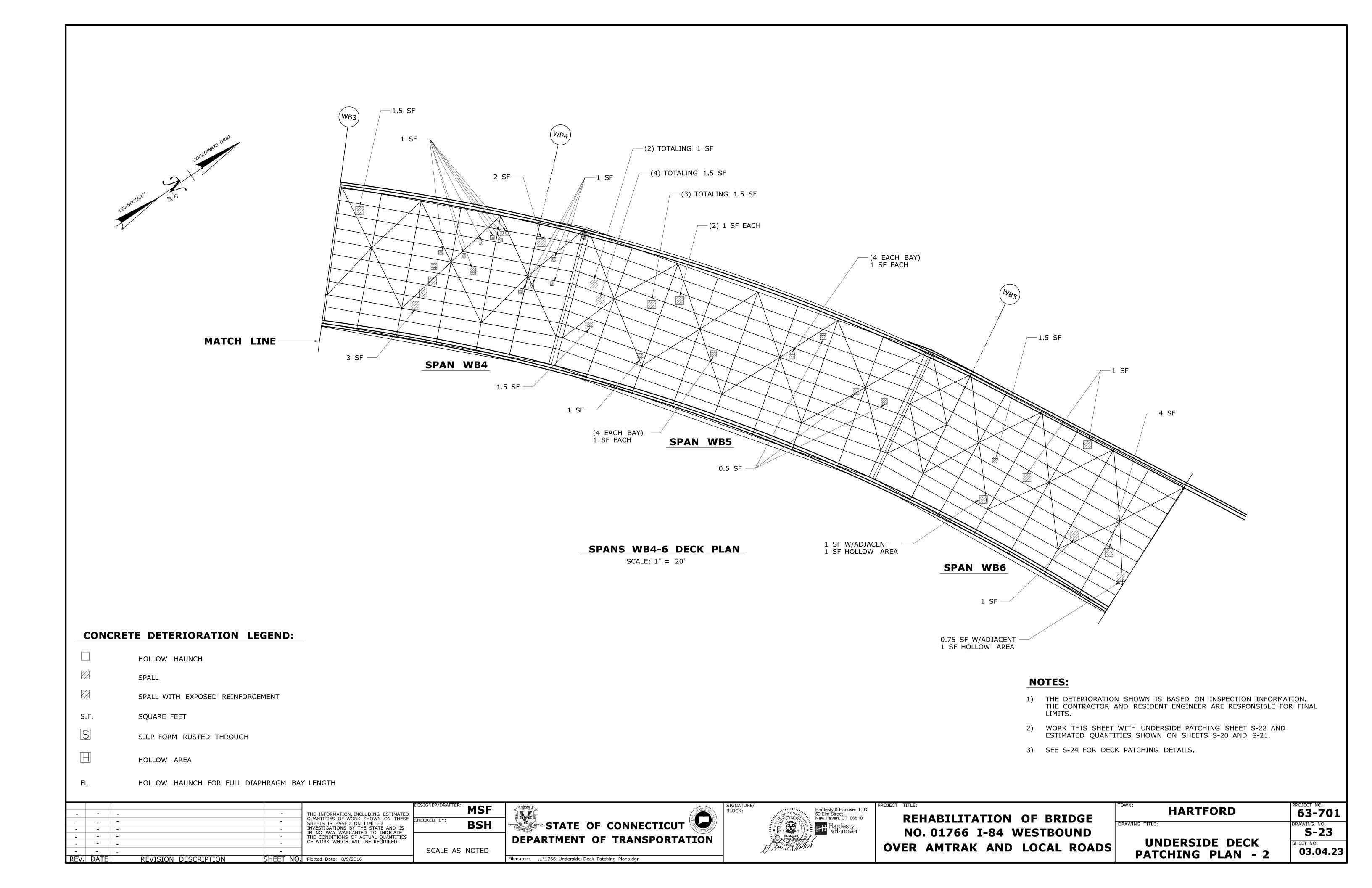
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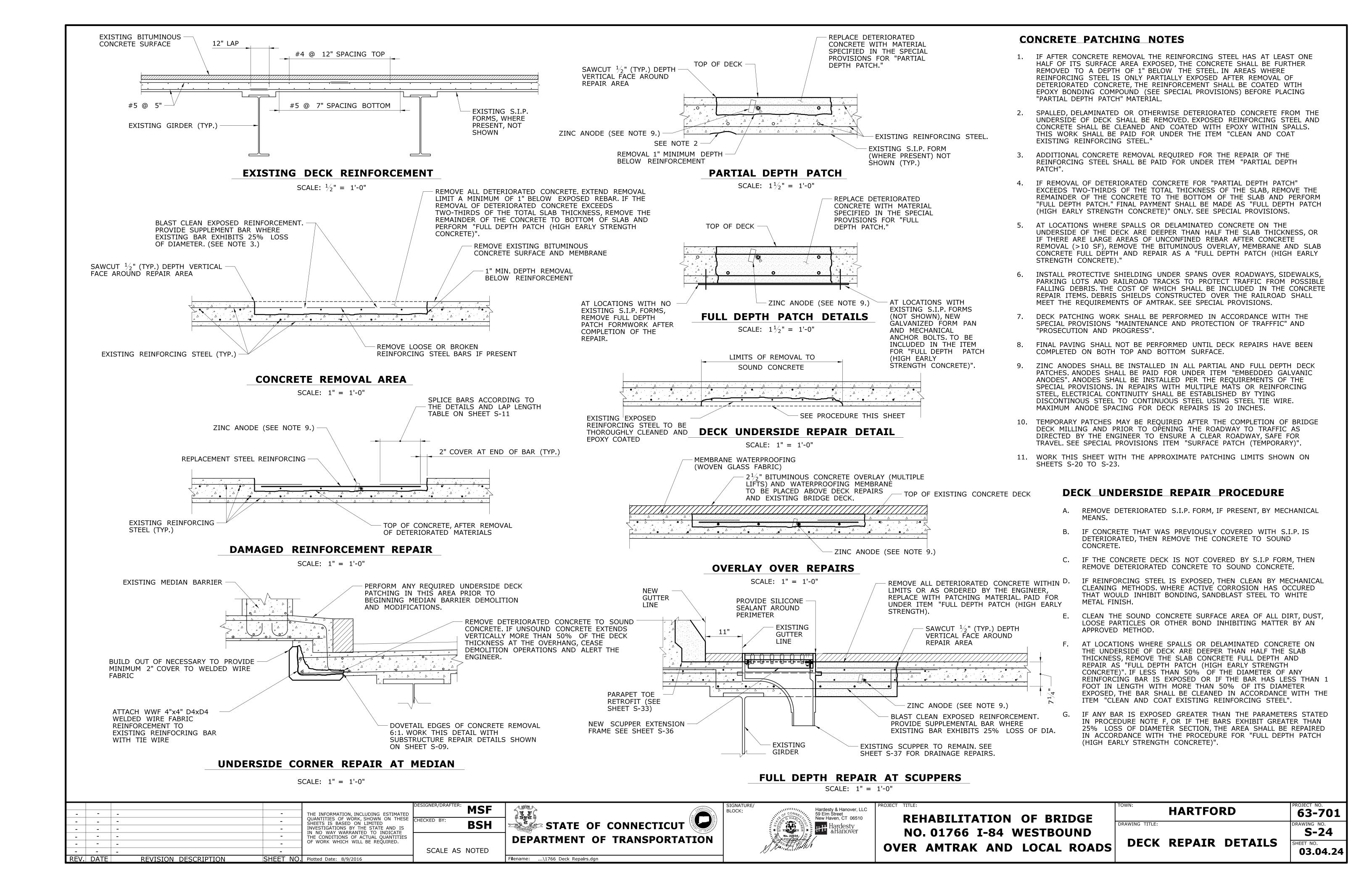
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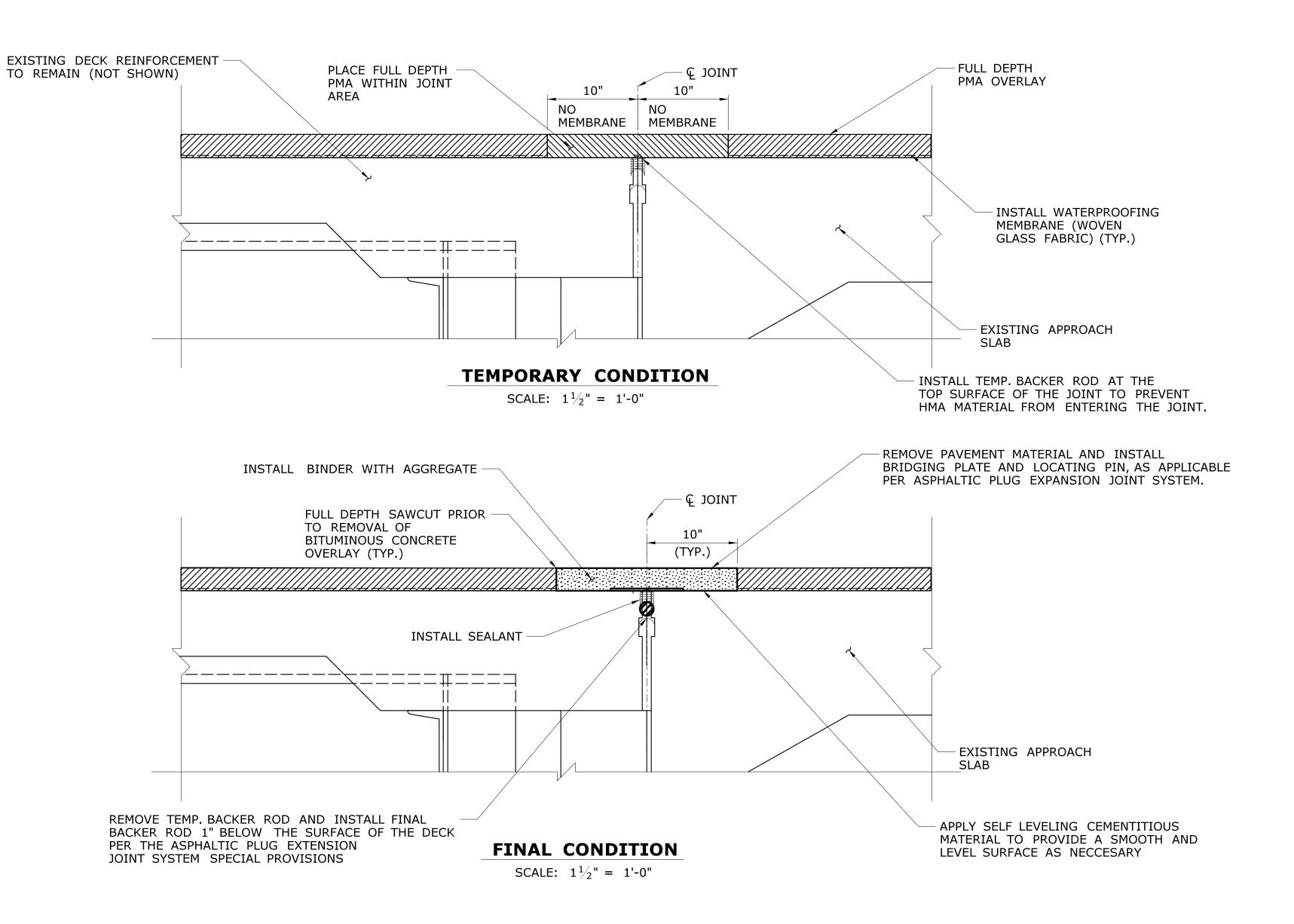


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	NO. 01766 I-84 WESTBOUND	
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HARTFORD	PROJECT NO. 63-701
DRAWING TITLE:	DRAWING NO. S-22
UNDERSIDE DECK PATCHING PLAN - 1	SHEET NO. 03.04.22







ASPHALTIC PLUG EXPANSION JOINT SYSTEM NOTES

- 1. BRIDGING PLATE ONLY USED AT JOINTS AT HANGER AND ABUTMENT 3-N. THE STEEL PLATES SHALL CONFORM TO THE REQUIREMENTS OF ASTM A709 GRADE 36. THE STEEL PLATES AND WELDED STUDS SHALL BE HOT DIPPED GALVANIZED IN CONFORMANCE WITH ASTM A123 AFTER FABRICATION.
- 2. THE REMOVAL OF ALL EXISTING JOINT SYSTEMS AND BITUMINOUS CONCRETE WITHIN THE LIMITS SHOWN SHALL BE PAID FOR UNDER THE ITEM "REMOVAL OF HMA WEARING SURFACE".
- 3. CRACK SEALANT PLACED ALONG VERTICAL FACES OF THE SAW-CUT PAVEMENT AND ON SURFACE AT JOINTS SHALL BE PAID UNDER THE ITEM "ASPHALTIC PLUG EXPANSION JOINT SYSTEM."
- 4. SAWCUTTING AND REMOVAL OF PAVEMENT FOR JOINT INSTALLATION SHALL BE PAID FOR UNDER THE ITEM "ASPHALTIC PLUG EXPANSION JOINT SYSTEM".
- 5. REFER TO SPECIAL PROVISIONS FOR ALLOWABLE BRIDGE SUPERSTRUCTURE TEMPERATURE RANGES DURING ASPHALTIC PLUG EXPANSION JOINT SYSTEM INSTALLATION.
- 6. SEALING OF PARAPET JOINTS IS PAID FOR UNDER THE ITEM "ASPHALTIC PLUG EXPANSION JOINT SYSTEM."
- 7. SEE TABLE THIS SHEET FOR THERMAL MOVEMENT RANGES.
- 8. THE CLOSED CELL BACKER ROD SHALL BE PLACED A MINIMUM OF 2" FROM THE OUTSIDE FACE OF PARAPETS AND MEDIAN BARRIERS, CLOSED CELL BACKER ROD DIAMETER, SHALL BE DETERMINED AFTER MEASURING THE JOINT OPENING, AND SHALL BE 25% LARGER THAN THE JOINT OPENING.
- THE NON-SAGGING SILICONE SEALANT SHALL BE REPLACED ON THE BACKER ROD 1/2" THICK. AT THE GUTTER, THE SILICONE SEALANT SHALL BE PLACED FLUSH WITH THE OUTSIDE FACE OF CONCRETE.
- 10. PRIOR TO INSTALLING THE SILICONE SEALANT, CLEAN JOINT SIDES BY SANDBLASTING. DUST SHALL BE REMOVED BY THE METHOD APPROVED BY THE ENGINEER. THIS WORK SHALL BE PAID FOR UNDER THE ITEM "ASPHALTIC PLUG EXPANSION JOINT SYSTEM"
- 11. SEE GENERAL PLAN SHEET S-02 FOR ASPHALTIC PLUG JOINT LOCATIONS.

ASPHALTIC PLUG EXPANSION JOINT SYSTEM - SUGGESTED SEQUENCE OF WORK:

- TEP 1 REMOVE THE EXISTING PAVEMENT MATERIAL AND THE JOINT MATERIAL.
- STEP 2 INSTALL TEMPORARY BACKER ROD FLUSH WITH THE BRIDGE DECK AND APPRAOCH SLAB.
- STEP 3 REPAIR DETORIORATED CONCRETE AS NEEDED TO BE PAID UNDER "PARTIAL DEPTH PATCH" OR "FULL DEPTH PATCH" ITEMS.
- STEP 4 INSTALL WATERPROOFING MEMBRANE (WOVEN GLASS FABRIC) TO THE TOP OF THE DECK AND APPROACH SLAB WITHIN THE LIMITS SHOWN, WHEN REQUIRED.
- STEP 5 PLACE PMA S0.25 AND PMA S0.50 (REFER TO BITUMINOUS CONCRETE PLACEMENT REQUIREMENTS NOTES ON S-03.
- STEP 6 CUT PAVEMENT FULL DEPTH AT 10" FROM THE CENTER OF THE JOINT (BOTH SIDES OF JOINT) AND REMOVE ALL PAVEMENT MATERIAL BETWEEN SAW-CUTS.
- STEP 7 INSTALL FINAL ASPHALTIC PLUG EXPANSION JOINT SYSTEM.

THE	THERMAL MOVEMENT RANGE*						
	JOINT OPENING						
	40°	50°	60°	70°	80°		
ABUTMENT 3-S	1 9/16 "	1 1/2"	1 7/16 "	13/8"	1 1/4"		
ABUTMENT 3-N	1 9/16 "	1½"	1 7/16 "	1 ⁵ / ₁₆ "	1 1/4"		

*JOINT OPENING AT 50° BASED ON ORIGINAL PLANS. CONTRACTOR TO VERIFY EXISTING JOINT OPENING AND MODIFY THERMAL MOVEMENT RANGE TABLE BASED ON FIELD OBSERVATIONS.

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SHEET NO. Plotted Date: 8/9/2016

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REHABILITATION OF BRIDGE NO. 01766 I-84 WESTBOUND OVER AMTRAK AND LOCAL ROADS

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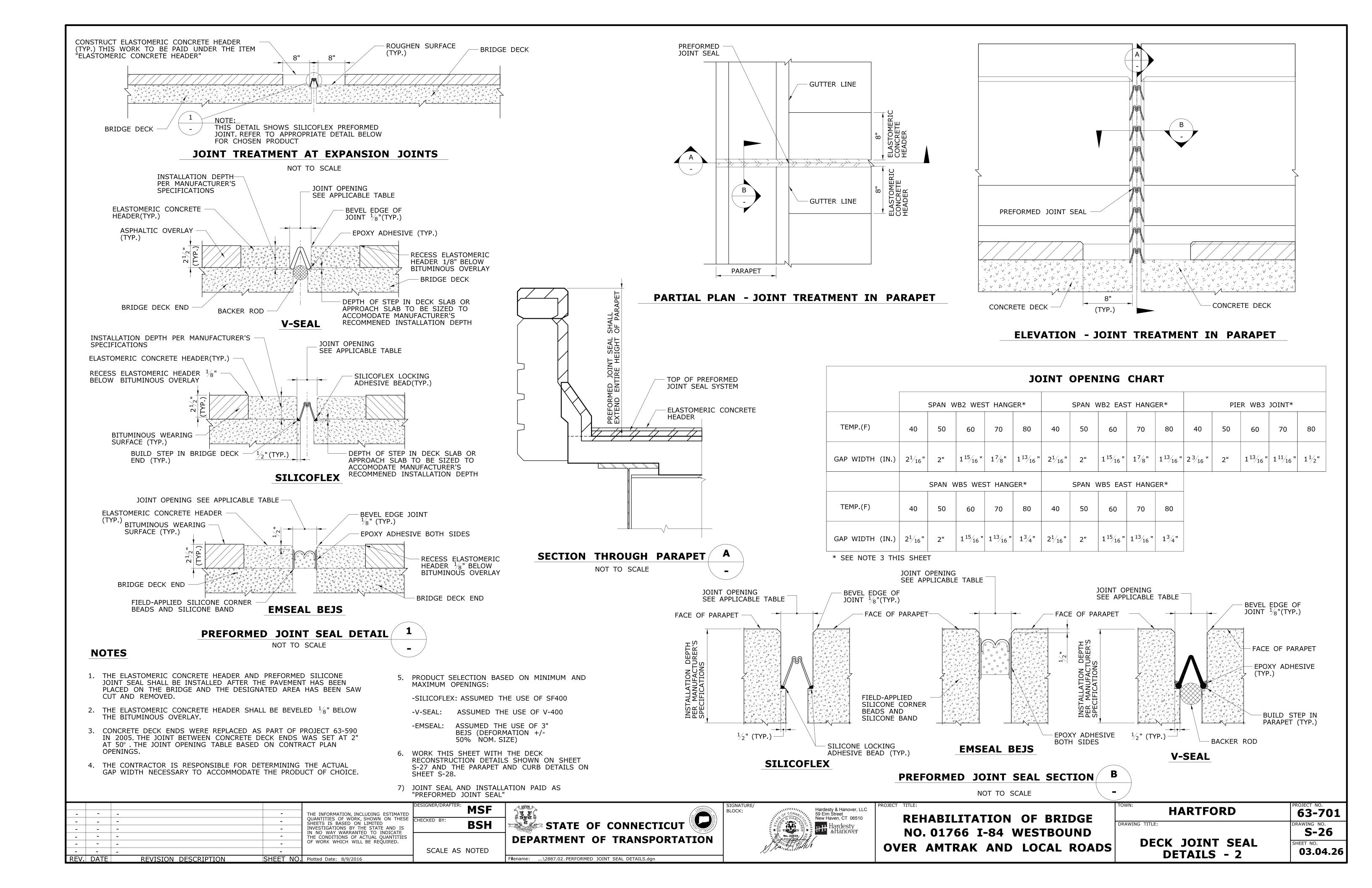
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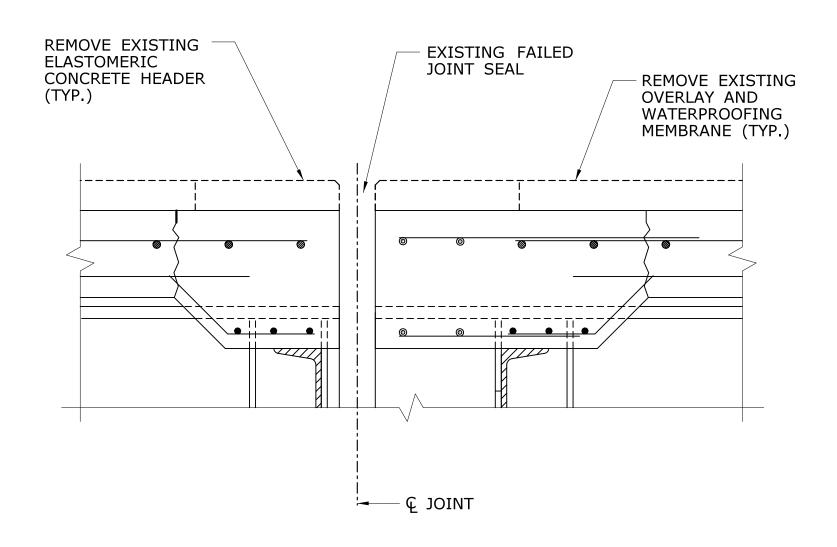
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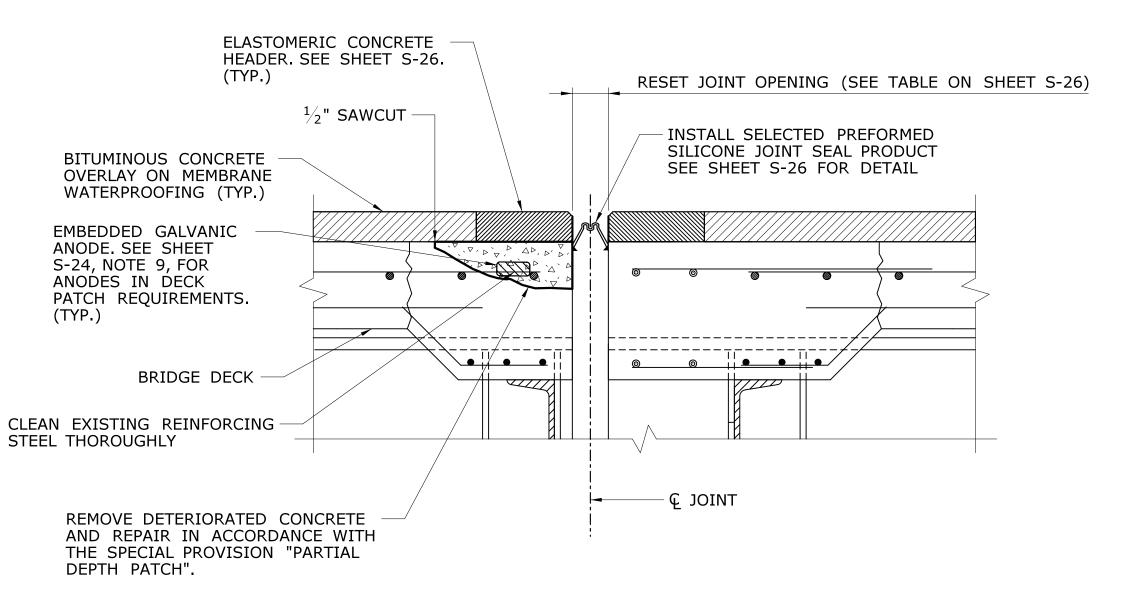
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DECK JOINT SEAL DETAILS - 1





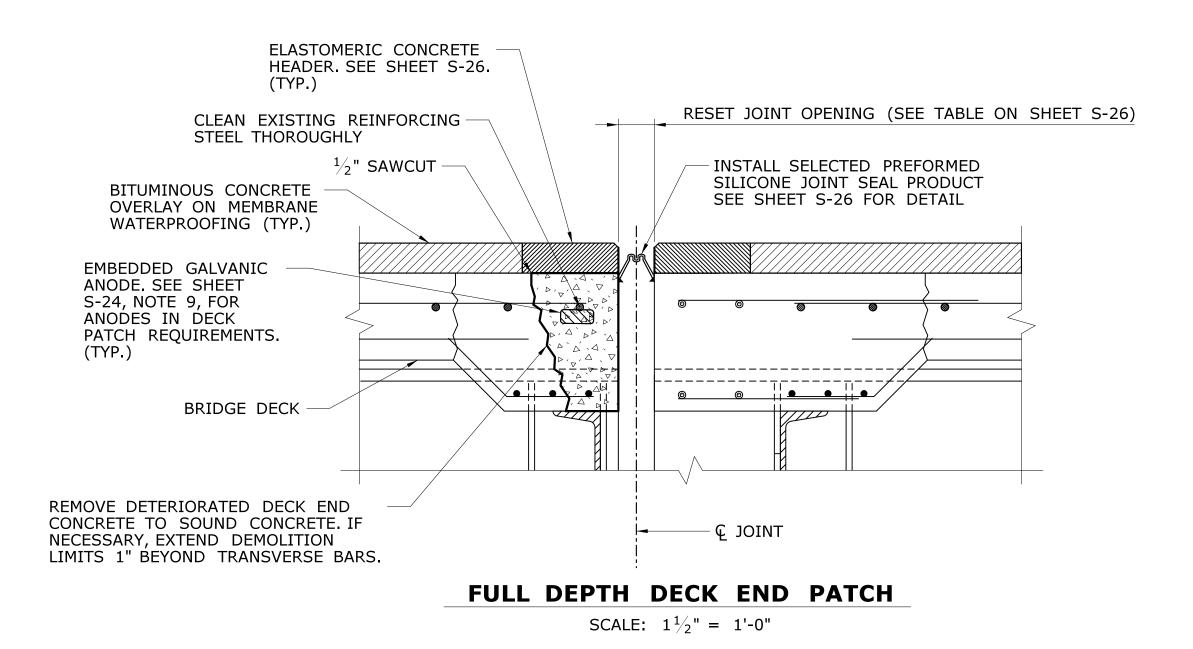


EXISTING EXPANSION JOINT

SCALE: $1\frac{1}{2}$ "=1'-0"

PARTIAL DEPTH DECK END PATCH

SCALE: $1\frac{1}{2}$ " = 1'-0"



NOTES

- 1. PARTIAL DEPTH DECK END PATCH SHALL BE PAID FOR UNDER THE ITEM "PARTIAL DEPTH PATCH". SEE SHEET S-24 AND SPECIAL PROVISION FOR PATCHING DETAILS AND PROCEDURE.
- 2. FULL DEPTH DECK END PATCH SHALL BE PAID FOR UNDER THE ITEM "FULL DEPTH PATCH (HIGH EARLY STRENGTH CONCRETE)". SEE SHEET S-24 AND SPECIAL PROVISION FOR PATCHING DETAILS AND PROCEDURE.
- 3. ANY DECK PATCHING AT DECK ENDS MUST BE COORDINATED WITH PARAPET AND CURB MODIFICATIONS. SEE SHEET S-28 FOR CURB AT JOINT DETAILS. SEE SHEET S-29 FOR PARAPET RETROFIT DETAILS.
- 4. REPAIRED DECK END WIDTH SHALL BE COORDINATED WITH THE PREFORMED SILICONE EXPANSION JOINT SYSTEM SELECTED. SEE SHEET S-26 FOR DETAILS AND JOINT OPENING TABLE.
- 5. COORDINATE DECK END REPAIR WITH PIER 3 CROSS FRAME MODIFICATIONS PERFORMED AT LOCATIONS OF STEEL KEEPER DEVICES. SEE SHEETS S-11 AND S-12.
- 6. REMOVE AND REPLACE ANY CORRODED REINFORCEMENT EXPOSED DURING DEMOLITION.
- 7. APPLY PRIME COAT TO ANY STRUCTURAL STEEL EXPOSED DURING DEMOLITION.

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REHABILITATION OF BRIDGE NO. 01766 I-84 WESTBOUND OVER AMTRAK AND LOCAL ROADS

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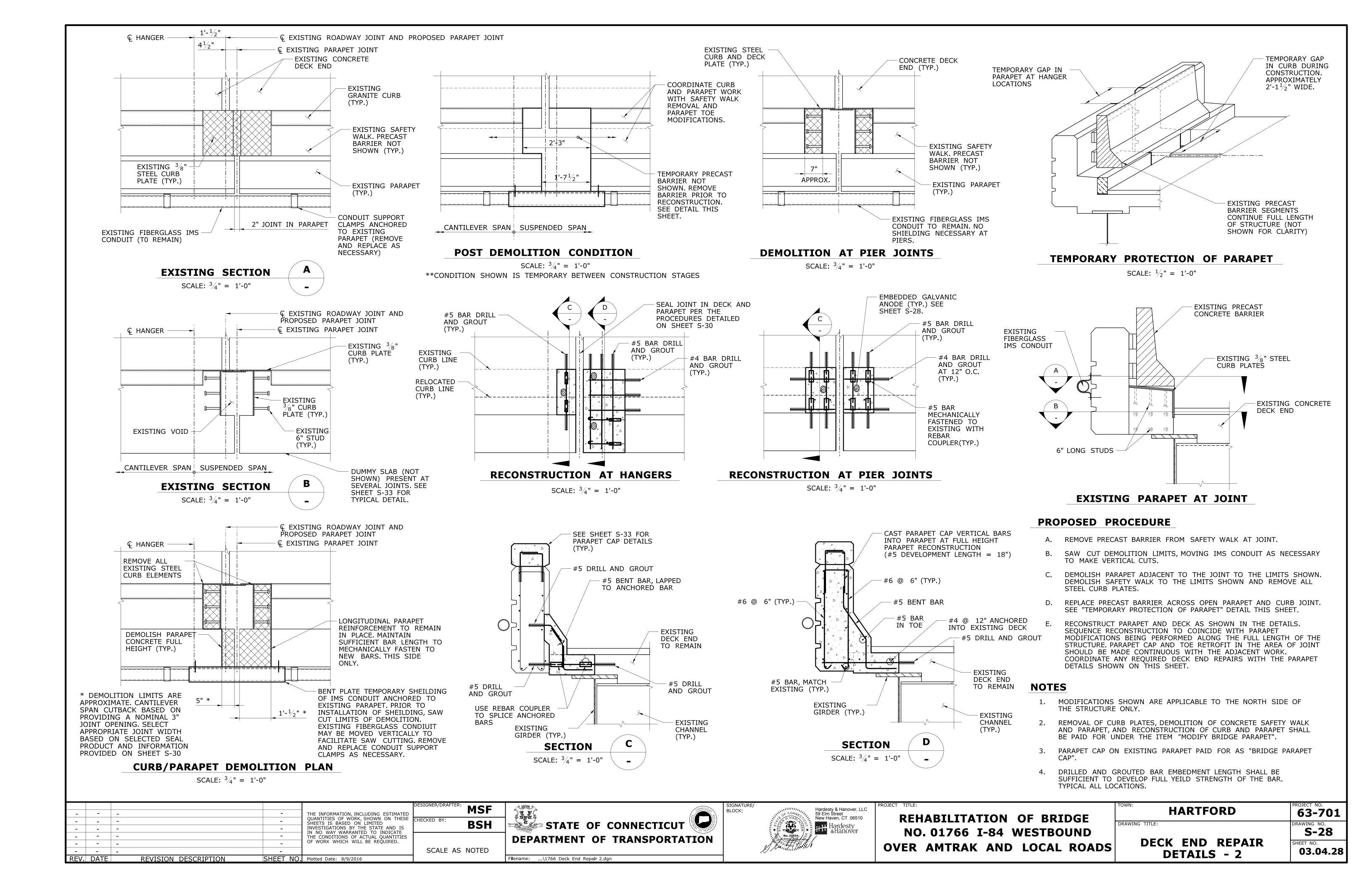
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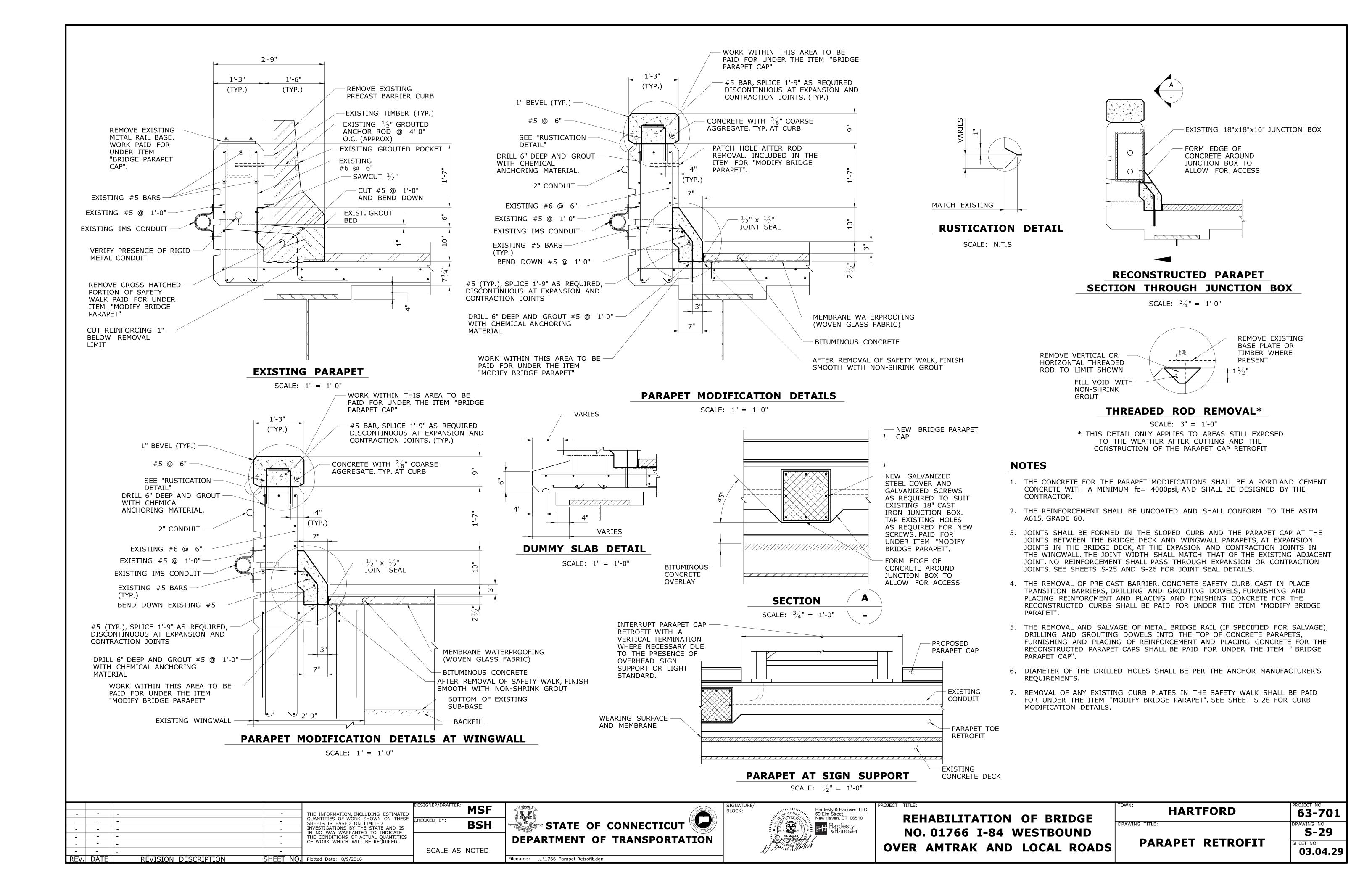
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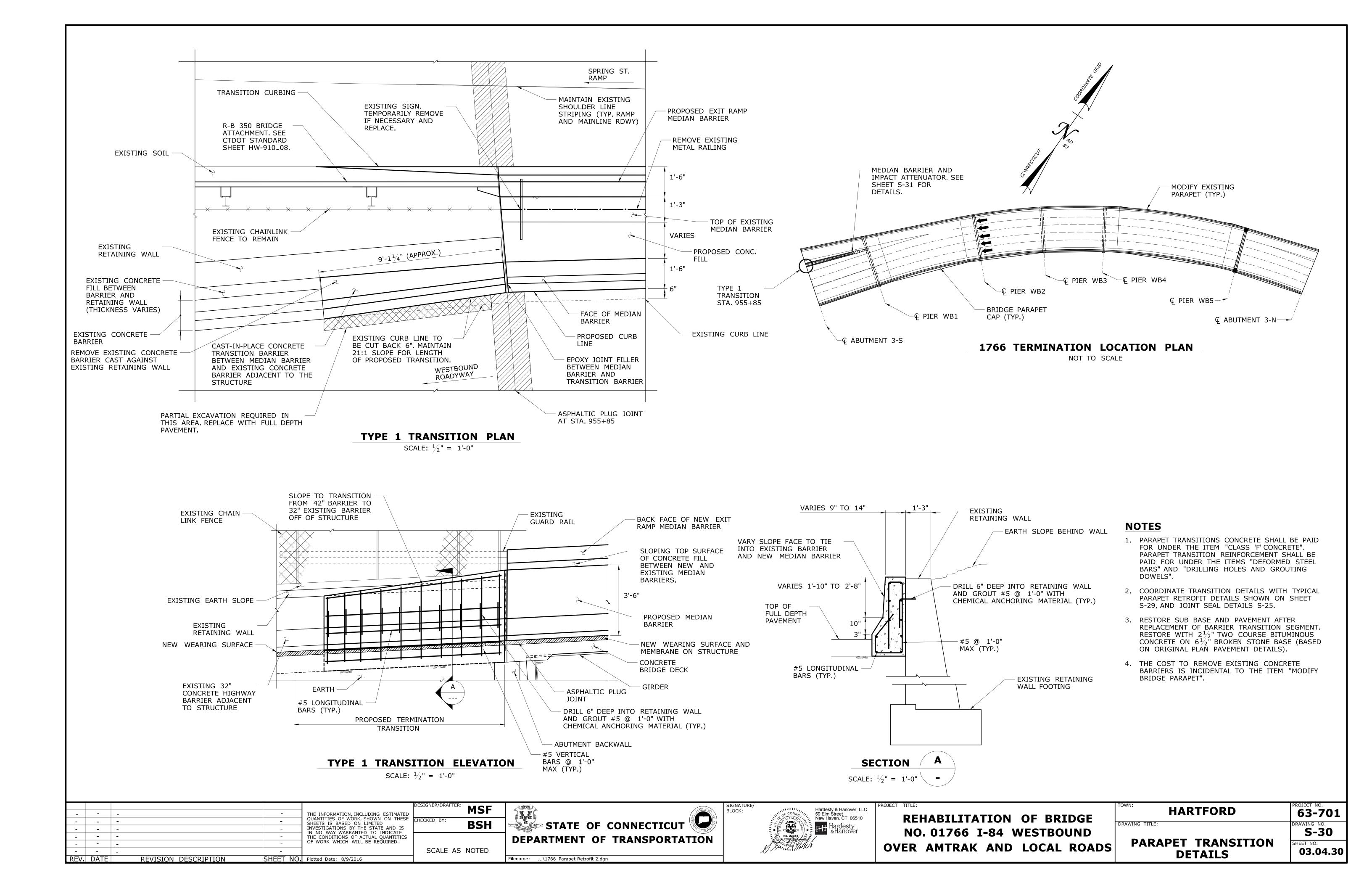
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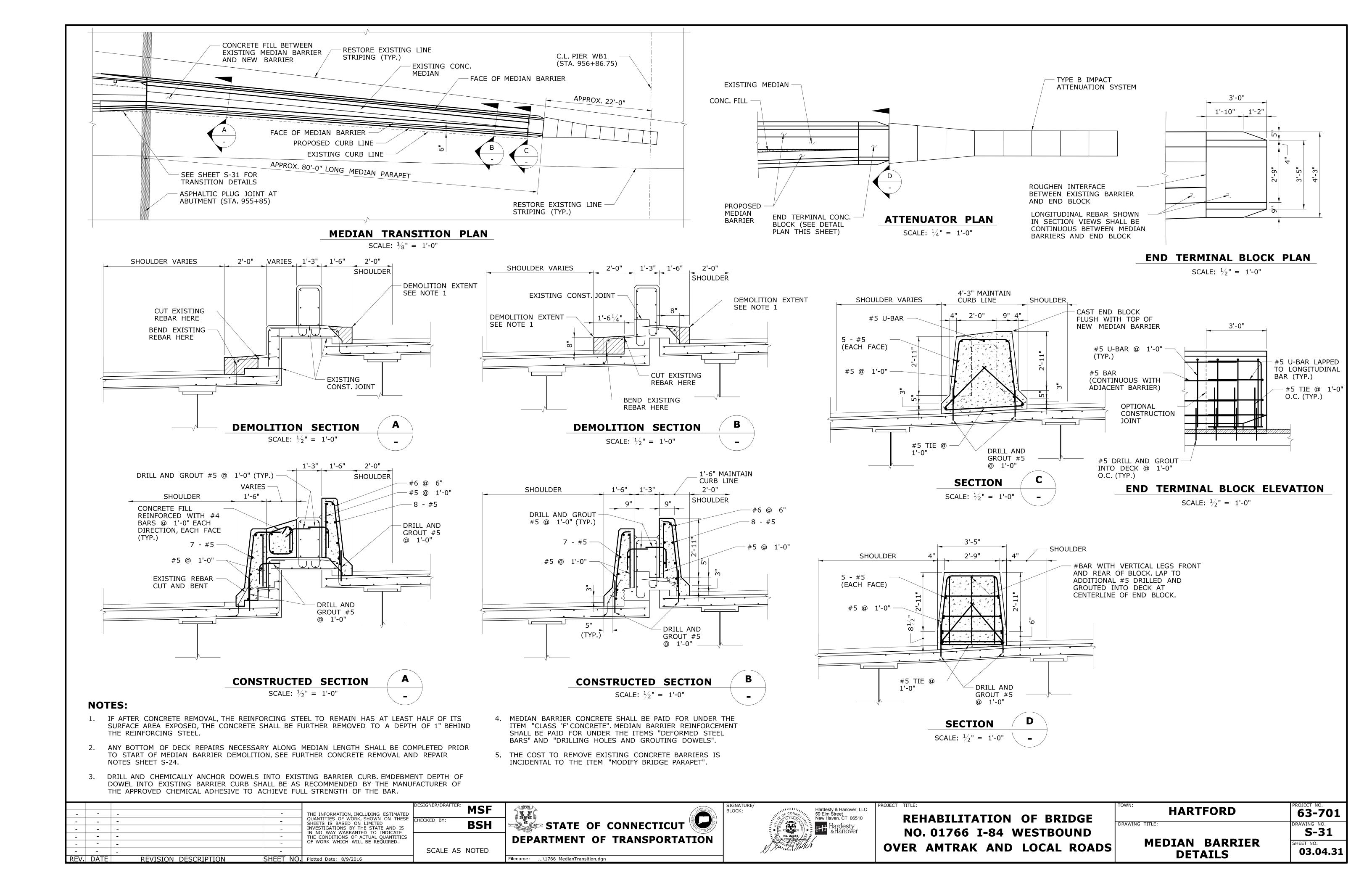
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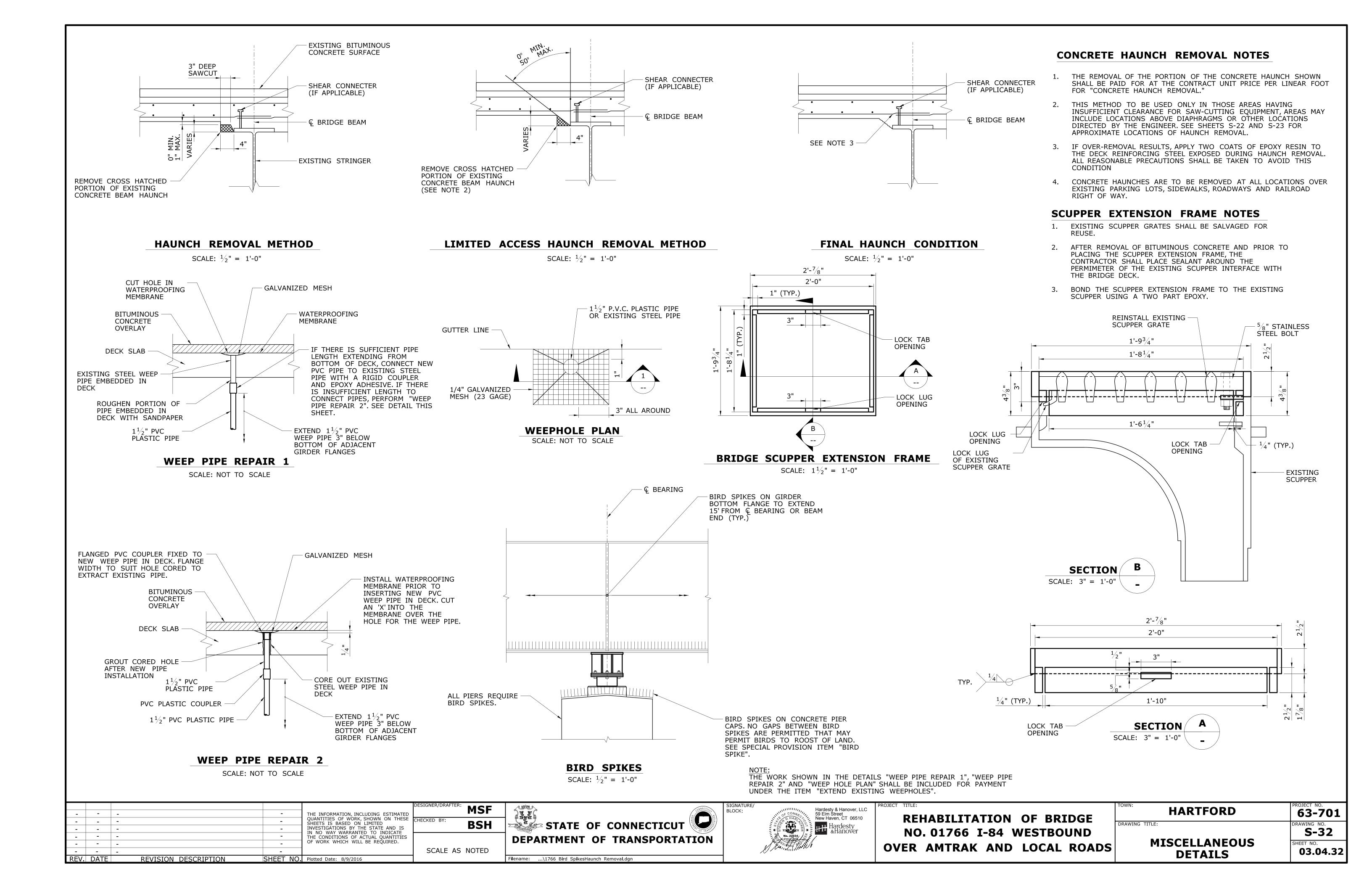
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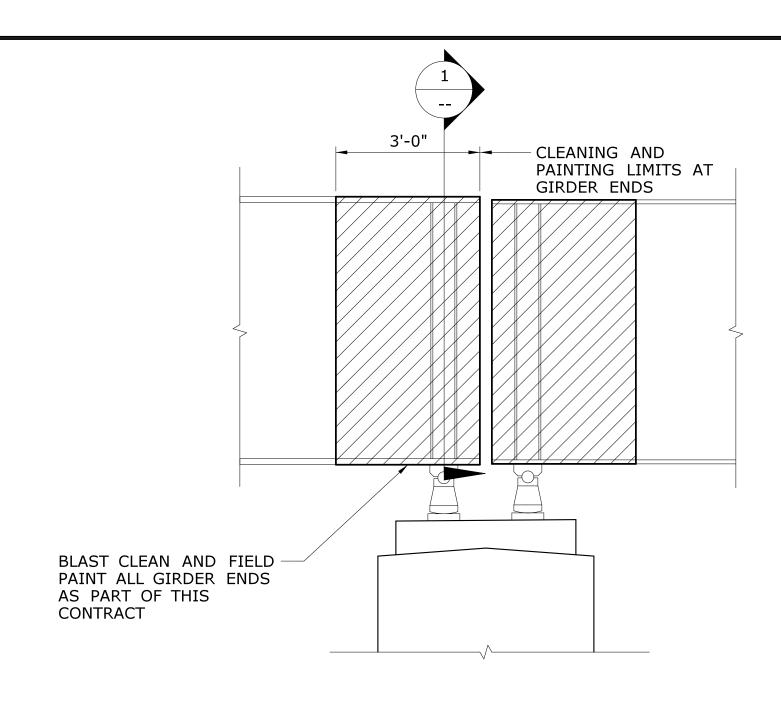


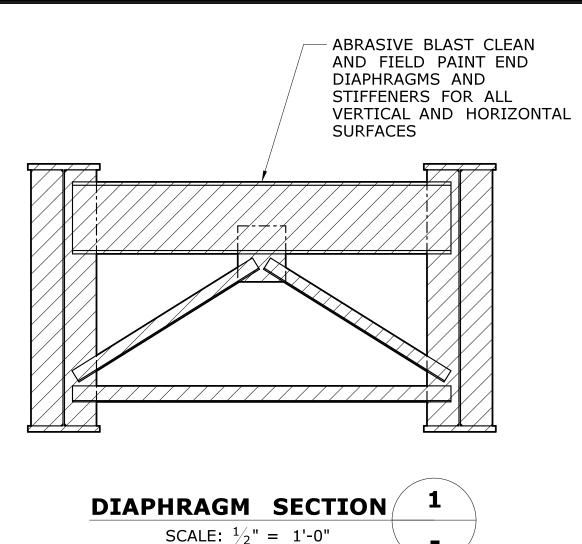






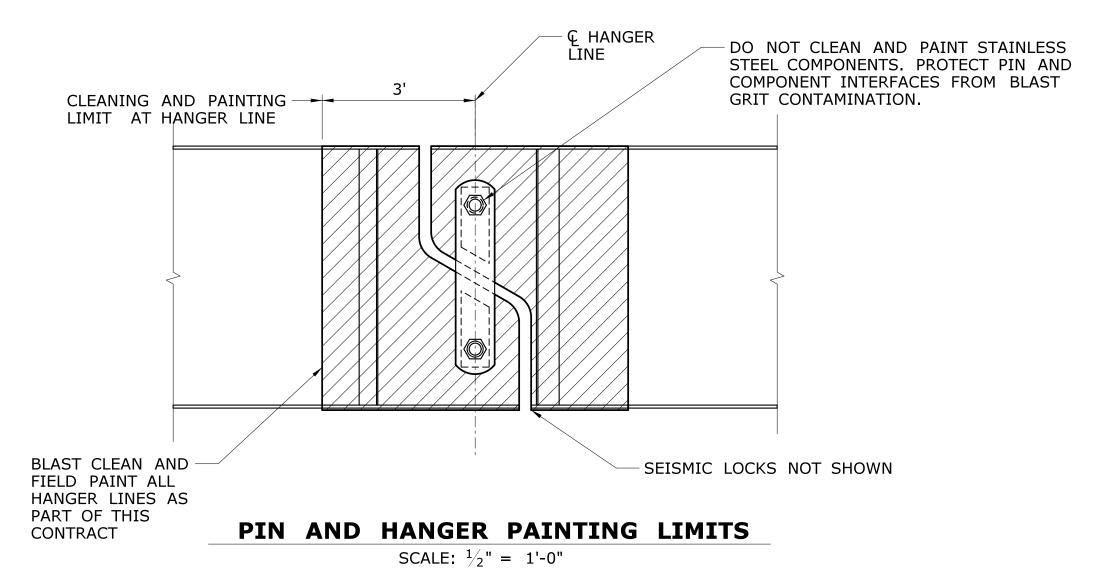






PARTIAL PAINTING LIMITS

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REHABILITATION OF BRIDGE NO. 01766 I-84 WESTBOUND OVER AMTRAK AND LOCAL ROADS

HARTFORD 63-701 DRAWING TITLE: **S-33**

PAINTING AND CONTAINMENT SHEET NO. 03.04.33

WORK SHIELDING AND PLATFORM NOTES:

- 1. THE CONTRACTOR SHALL PROVIDE PROTECTIVE BARRIERS FOR ALL WORK ABOVE ACTIVE ROADWAYS, SIDEWALKS, PARKING AREAS, AND AMTRAK RAILROAD.
- 2. WORK OVER THE RAILROAD SHALL BE STAGED IN COMPLIANCE WITH A SITE SPECIFIC WORK PLAN PREPARED BY THE CONTRACTOR FOR THE APPROVAL OF THE RAILROAD. WHERE WORK IS STAGED FROM RIGID WORK LATFORMS AND CONTAINMENT THAT IS CONNECTED TO THE STRUCTURE ABOVE, SUCH PLATFORMS SHALL MEET HE RAILROAD'S REQUIREMENTS AND SHALL BE SUBMITTED FOR THE RAILROAD'S APPROVAL.
- 3. THE CONTRACTOR'S WORKING DRAWINGS FOR SHIELDS OVER THE RAILROAD SHALL INCLUDE ALL MINIMUM PROPOSED VERTICAL CLEARANCES TO THE TOP OF RAIL
- 4. WORK OVER AREAS OTHER THAN THE RAILROAD SHALL BE STAGED IN ACCORDANCE WITH WORK PLANS DEVELOPED FOR THE REVIEW AND APPROVAL OF THE RESIDENT ENGINEER, SEE SPECIAL PROVISIONS FOR REQUIREMENTS.
- TEMPORARY PROTECTIVE BARRIERS OVER THE RAILROAD SHALL BE CONSIDERED INCIDENTAL TO WORK NECESSARY. THESE BARRIERS SHALL MEET THE RAILROAD'S REQUIREMENTS FOR HORIZONTAL AND VERTICAL SHIELDING. SEE SPECIAL PROVISIONS.
- 6. TEMPORARY PROTECTIVE BARRIERS ARE INCIDENTAL TO THE WORK FOR WHICH THEY ARE REQUIRED.
- 7. WHERE ABRASIVE BLAST CLEANING AND FIELD PAINTING IS TO BE PERFORMED, THE PLATFORMS SHALL SATISFY THE REQUIREMENTS FOR "CLASS 1 CONTAINMENT AND COLLECTION OF SURFACE PREPARATION DEBRIS (SITE No. 3)", AND SHALL BE INCLUDED FOR PAYMENT AS NOTED THEREIN. WHERE THE WORK IS TO BE PERFORMED OVER THE RAILROAD, THE PLATFORMS SHALL MEET ADDITIONAL HORIZONTAL AND VERTICAL SHIELDING REQUIREMENTS AS IDENTIFIED IN THE SPECIAL PROVISIONS.

ABRASIVE BLAST CLEANING AND FIELD PAINTING NOTES:

- 1. THIS STRUCTURE WAS ORIGINALLY COATED WITH A LEAD BASED PAINT SYSTEM.
- 2. THE EXISTING COATING SHALL BE REMOVED IN THE AREAS NOTED PRIOR TO THE APPLICATION OF THE NEW COATING SYSTEM.
- REMOVAL OF THE EXISTING LEAD BASED PAINT SYSTEM REQUIRES WORK PLATFORMS MEETING THE REQUIREMENTS IN THE SPECIAL PROVISION FOR "CLASS 1 CONTAINMENT AND COLLECTION OF SURFACE PREPARATION DEBRIS".
- 4. ABRASIVE BLAST CLEANING, FIELD PAINTING, AND DISPOSAL OF LEAD DEBRIS SHALL BE IN ACCORDANCE WITH THE SPECIAL PROVISIONS AND ALL FEDERAL, STATE, AND LOCAL REGULATIONS.
- THE CONTRACTOR SHALL IMPLEMENT A SITE SPECIFIC LEAD COMPLIANCE PLAN PREPARED BY A CERTIFIED INDUSTRIAL HYGENIST IN ACCORDANCE WITH THE SPECIAL PROVISIONS.
- AFTER ABRASIVE BLAST CLEANING AND BEFORE THE APPLICATION OF THE PRIME COAT, THE CONTRACTOR SHALL PROVIDE ACCESS TO THE ENGINEER FOR THE INSPECTION OF THE EXISTING STEEL TO DETERMINE REMAINING THICKNESS.
- 7. THIS WORK SHALL BE GOVERNED BY THE SPECIAL PROVISION FOR "ABRASIVE BLAST CLEANING AND PAINTING OF BEAM ENDS (SITE No. 3)".
- 8. THE SPECIAL PROVISION FOR "LOCALIZED PAINT REMOVAL & FIELD PAINTING OF EXISTING STEEL" SHALL GOVERN ADDITIONAL AREAS WHERE PAINT REMOVAL AND RECOATING IS INCIDENTAL TO OTHER WORK ITEMS.

CONTAINMENT NOTES:

- THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND CONSTRUCTION OF THE PLATFORMS IN COMPLIANCE WITH THE RAILROAD'S SPECIFICATIONS. THE CONTRACTOR SHALL SUBMIT, FOR RAILROAD APPROVAL, CALCULATIONS AND DETAILED WORKING DRAWINGS FOR THE CONTAINMENT SYSTEM, DESIGN CALCULATIONS OF THE WORK PLATFORM SHALL INCLUDE LOCATION OF PLATFORM SUPPORTS AND LOADING WHICH SHALL NOT PRODUCE A LOADING CONDITION THAT MAY OVERSTRESS THE STRUCTURE. SEE SPECIAL PROVISIONS.
- 2. DESIGN LOADS SHALL BE GOVERNED BY THE RAILROAD REQUIREMENTS (I&C SPECIFICATION 01520A-1 SECTION 3.1E). DESIGN WIND LOAD IS 30 PSF.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR LABOR AND EXPENSES RELATED TO COORDINATION WITH THE RAILROAD DURING ALL FIELD ACTIVITIES, INCLUDING THE WORK TO SECURE ACCESS PERMITS AND FLAG PROTECTION DURING THE PERIODS THAT THE CONTAINMENT IS IN PLACE AND OCCUPIED.
- 4. RIGID CONTAINMENT OCCUPANCY SHALL BE GOVERNED BY THE RAILROAD REQUIREMENTS.
- 5. WHEN WIND SPEED EXCEEDS 30 MPH ALL WORK SHALL STOP. DUST AND SAND SHALL BE REMOVED FROM THE PLATFORM. WHEN WIND SPEED EXCEEDS 40 MPH, ALL ENCLOSURE CONTAINMENT AND TARPS SHALL BE REMOVED FROM THE PLATFORMS.
- ABRASIVE AND WASTE DEBRIS SHALL BE REMOVED AS REQUIRED AND/OR ON A DAILY BASIS SO AS NOT TO EXCEED THE CAPACITY OF THE STRUCTURE OR PLATFORM.
- CONSTRUCTION AND ERECTION OF THE WORK PLATFORM AND CONTAINMENT STRUCTURE SHALL BE SCHEDULED TO COMPLY WITH RAILROAD REQUIREMENTS.
- CONTAINMENT INCLUDED FOR PAYMENT UNDER THE ITEM "ABRASIVE BLAST CLEANING AND FIELD PAINTING OF BEAM ENDS (SITE No. 3)".

